

FRONT



SIDE 1



SIDE 2



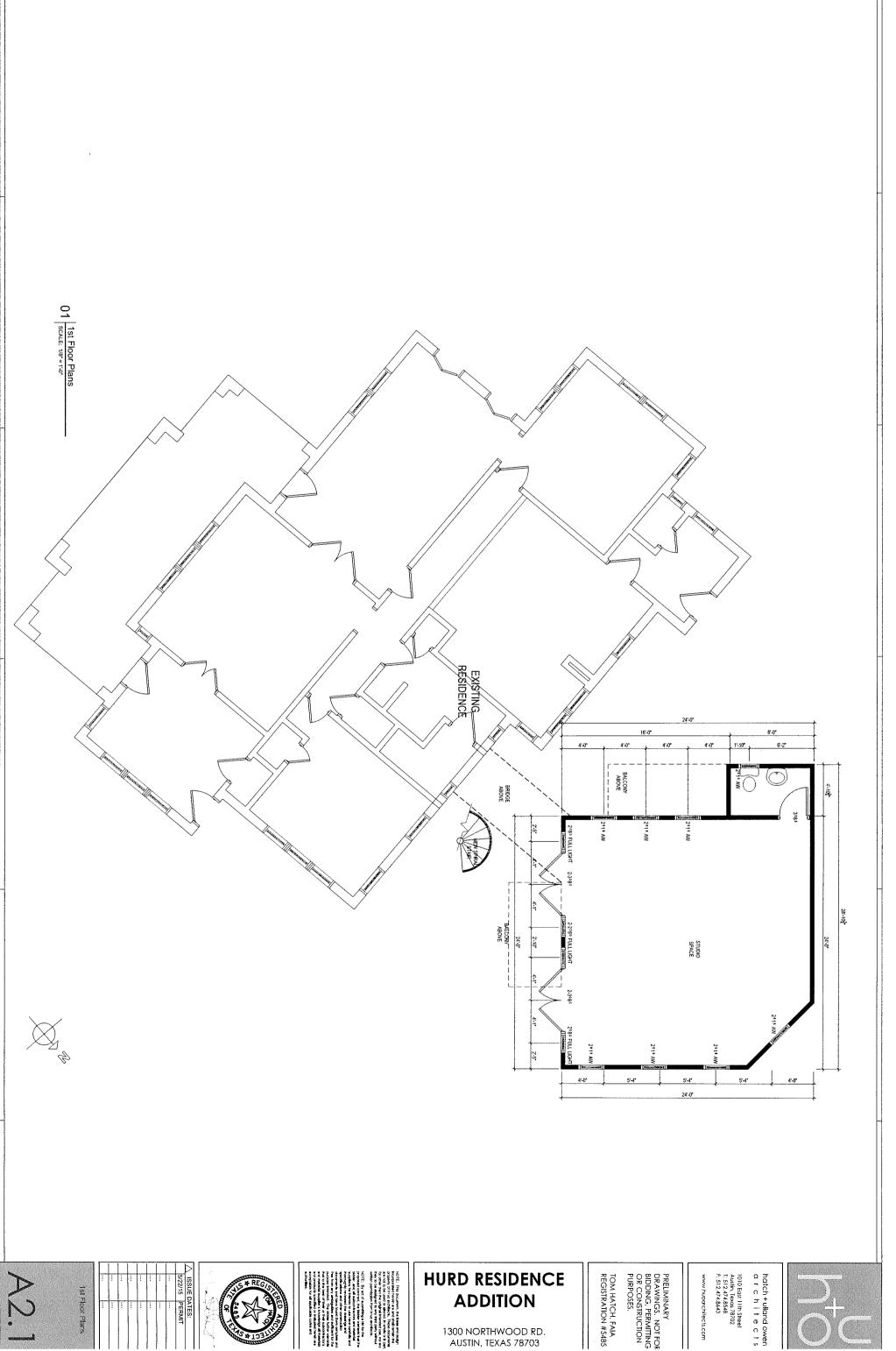
SIDE 3



2ND FLOOR WINDOW/WALL DEMO FOR BRIDGE

**BUILDING COVERAGE** IMPERVIOUS COVERAGE Ξ F. GROSS FLOOR AREA AND FLOOR AREA RATIO 2nd Floor Conditioned Area TOTAL Wood Decks ( must be counted at 100%) 1st Floor Conditioned Area Other: Concrete Steps Uncovered Wood Decks (may be counted at 50%) Other Building or Covered Areas (if applicable) Covered Patios Garage / Carport 3rd Floor Conditioned Area 2nd Floor Gross Area Air Conditioner Pad(s) Uncovered Patios Swimming Pool (surface area) Basement Gross Area 3rd Floor Gross Area TOTAL IMPERVIOUS COVERAGE (add a. through h.) Concrete Decks Sidewalk / Walkways on Private Property Breezeways FLOOR AREA RATIO (gross floor area /gross area of lot) TOTAL GROSS FLOOR AREA GROSS AREA OF LOT TOTAL BUILDING COVERAGE ON LOT TOTAL BUILDING AREA (add a. through I.) Covered Porches k. Attached
 (subtract 200 sq. ft. if used to meet the minimum parking requirement)
 1. Detached otal Building Coverage on Lot (see above b. 1st Floor Area with Ceiling Height Over 15 Feet. h. 3rd Floor Area with Ceiling Height Over 15 Feet. g. 3rd Floor Area (including all areas covered by a roof) d. 2nd Floor Area (including all areas covered by a roof) a. 1st Floor Area (excluding covered or uncovered finished ground-floor porches) e. 2nd Floor Area with Ceiling Height Over 15 Feet. TOTAL (add g and h above) Floor area outside footprint of first floor TOTAL (add d and e above) TOTAL (add a and b above) Attached Garage (Unit B) ibtract 450 square feet if more than 10 feet from principle structure) vithout habitable space above it subtract 450 sq. ft.) EXISTING 3,114 S.F. 1829 S.F. 975 S.F. N/A N/A N/A N/A N/A NA NA N/A 58+57=115 N/A NEW / ADDITION 2,755 S.F. 21.7% OF LOT 4,319 S.F. 1,398 S.F. 2,755 S.F. 1,225 S.F. 302 S.F. 18 S.F. 667 S.F. 19 S.F. NIA NIA NIA N/A N/A N. N/A EXISTING 2804 S.F. 975 S.F. N/A 1829 S.F. N/A 1829 S.F N/A N/A N N N NEW / ADDITION 4,087 S.F. 12,710 S.F. 1,283 S.F. 667 S.F. N/A 667 S.F. 616 S.F. 616 S.F. N A N/A N/A A/A clearance from AE energized power lines. Enforced by AE & NESC codes. All structures etc. must maintain 75" AUSTIN WATER UTILITY
CONSUMER SERVICE DIVISION - TAPS AE APPROVED 610.4 609.4 HIGH POINT JUL 0 8, 2015 JUL 0 8 2015 RLS 1891 608.4-HIGH POINT TENT2 607.7-HIGH POINT TENT 3 G TREE LIST A 14" PECAN 40" OAK 53" OAK 34" OAK 17" PECAN 14" PECAN 17" PECAN 15" PECAN 15" PECAN 18" PECAN 14" PECAN 511 515.0 AC. of) ADDITION AIC UNIT UTILITY POLE EXISTING GRADE ELECTRICAL LINES REQUIRED SETBACKS BUILDING FOOTPRINT PROPOSED SPOTELEV vc) EXISTING HOUSE 3AORY NOOTN'S ZONING: FRONT YARD SETBACK: SIDE YARD SETBACK: DRTHWOOD ROAD Second Floor Conditioned Area: First Floor Conditioned Area: Second Floor Conditioned Area: MAXIMUM BUILDING COVERAGE MAXIMUM IMPERVIOUS COVERAGE EXISTING HOUSE First Floor Conditioned Area: ADDITION Total Conditioned Area: REAR YARD SETBACK Total Conditioned Area: AND 25' SETBACK (60, 848 1829 S.F. 975 S.F. 2804 S.F. SF-3-NP 25 FEET 5 FEET 10 FEET 616 S.F. 667 S.F. 1283 S.F. 40% 45% TENT1 P.O. 2. 01 Architectural Site Plan EXISTING DRIVE MIERAGE ADJACENT GRADE = 807.5
IOM PORIT ADJACENT GRADE = 807.5
HIGH POINT ADJACENT GRADE = 810.0
807.5 - 610.0=1217.5
1217.67.2 = 868.75
AVERAGE ADJACENT GRADE = 808.8 A1.0 Architectural Site Plan & Project Information
A2.1 1st Fboor Plans
A2.2 2nd Fboor Plans
A2.3 RCP & Power Plan - Addition
A2.4 Roof Plans
A2.4 Roof Plans
A3.1 Exterior Elevations - Addition
A3.2 Exterior Elevations - Addition
A3.3 Exterior Elevations - Existing
A3.4 Exterior Elevations - Existing
A3.4 Exterior Elevations - Existing DRAWING INDEX Architectural Site Plan Project Information PRELIMINARY
DRAWINGS. NOT FOR
BIDDING, PERMITTING
OR CONSTRUCTION
PURPOSES. **HURD RESIDENCE** 1010 East 11th Street Austin, Texas 78702 T: 512.474.8548 F: 512.474.8643 hatch+ulland owen architects TOM HATCH, FAIA REGISTRATION #5485 **ADDITION** 

1300 NORTHWOOD RD. AUSTIN, TEXAS 78703







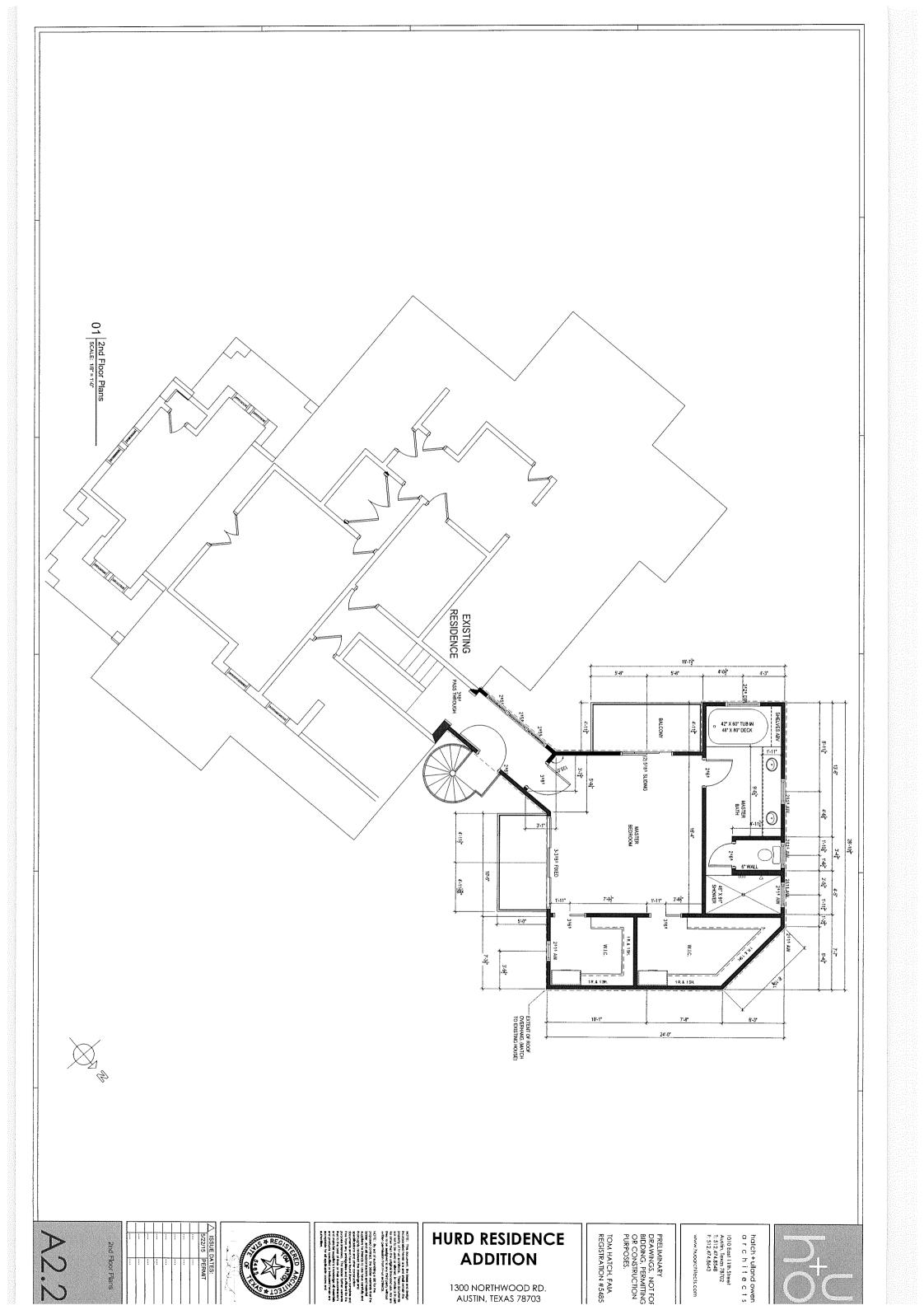


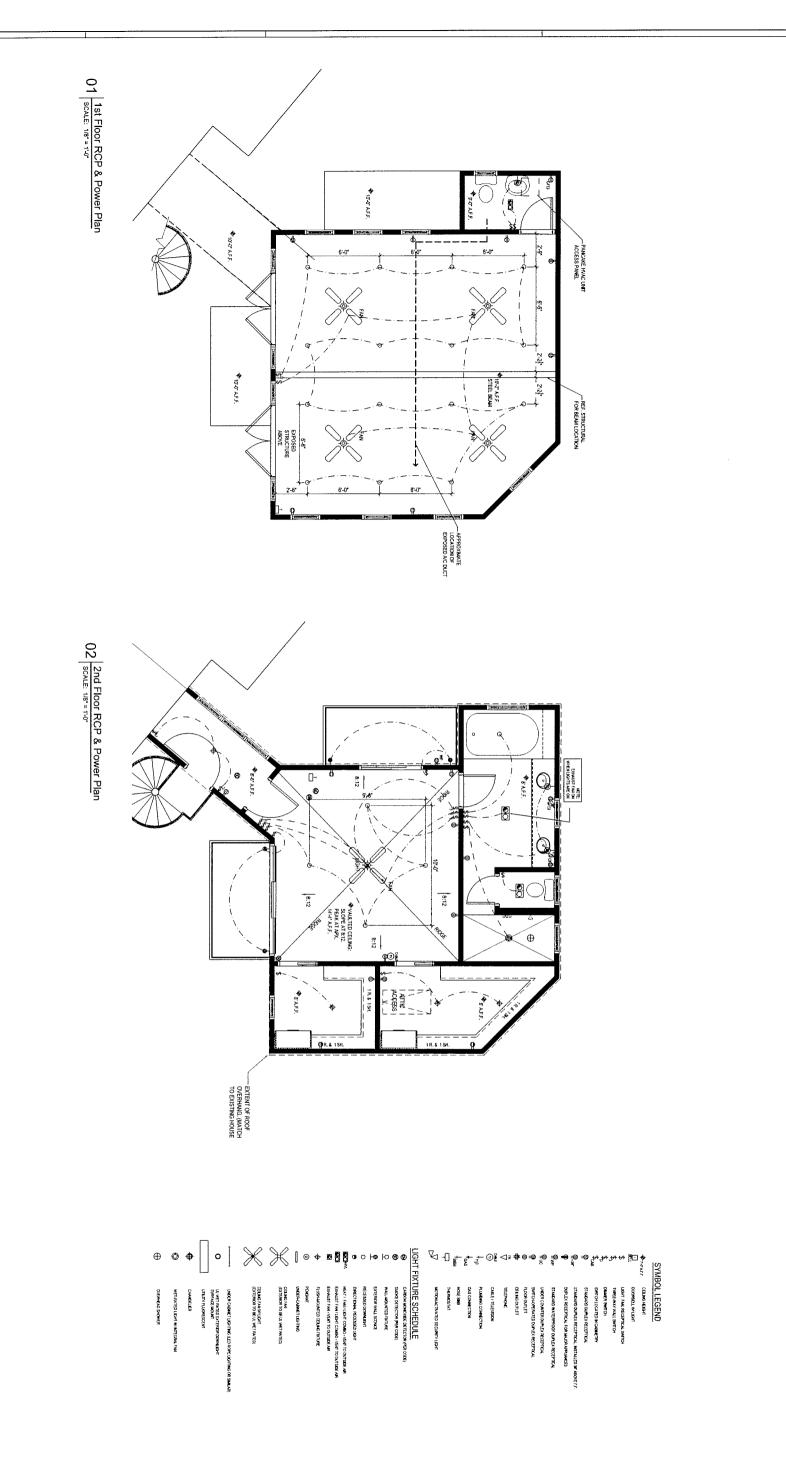


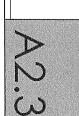
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OR CONSTRUCTION
PURPOSES.













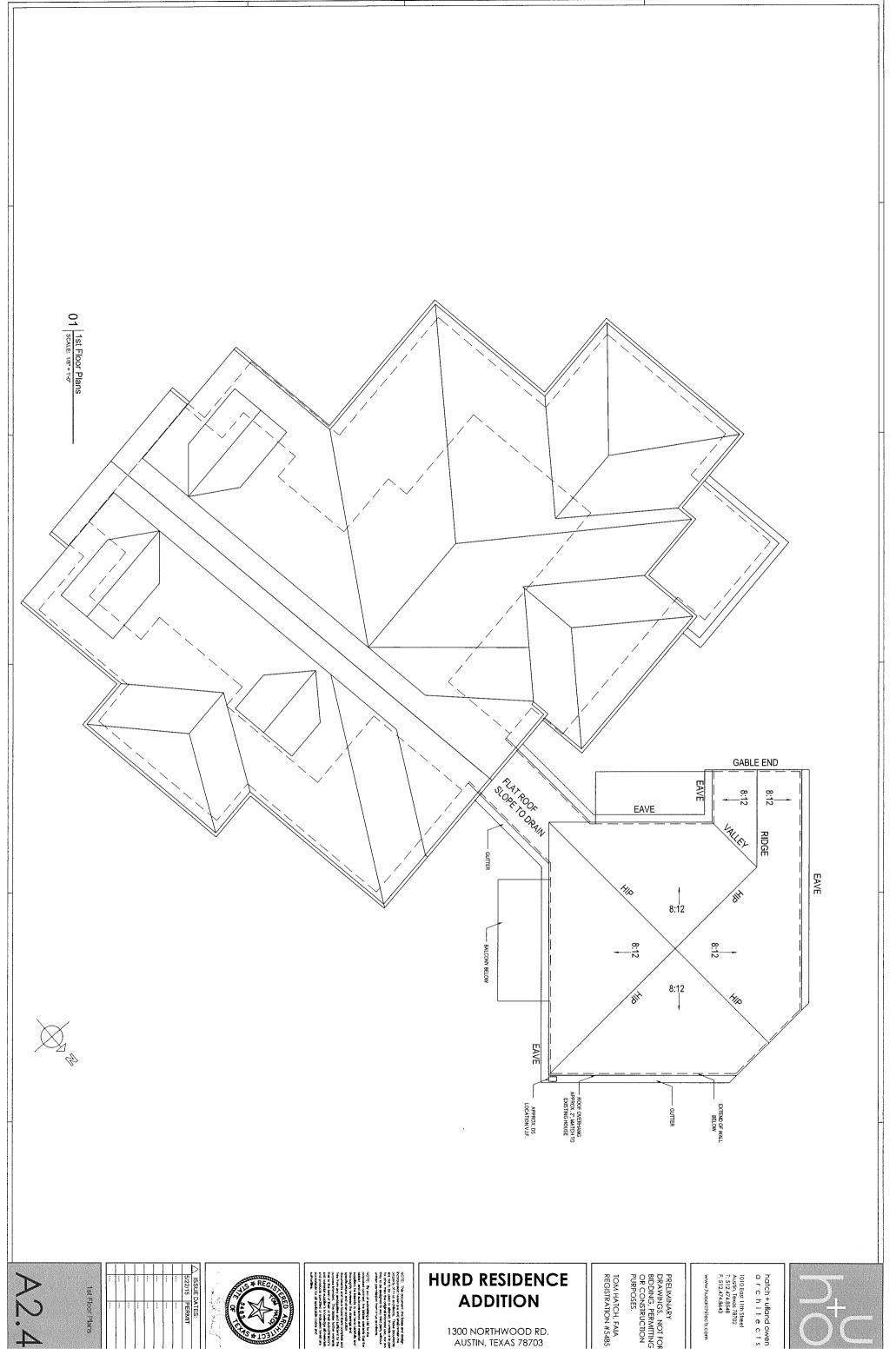




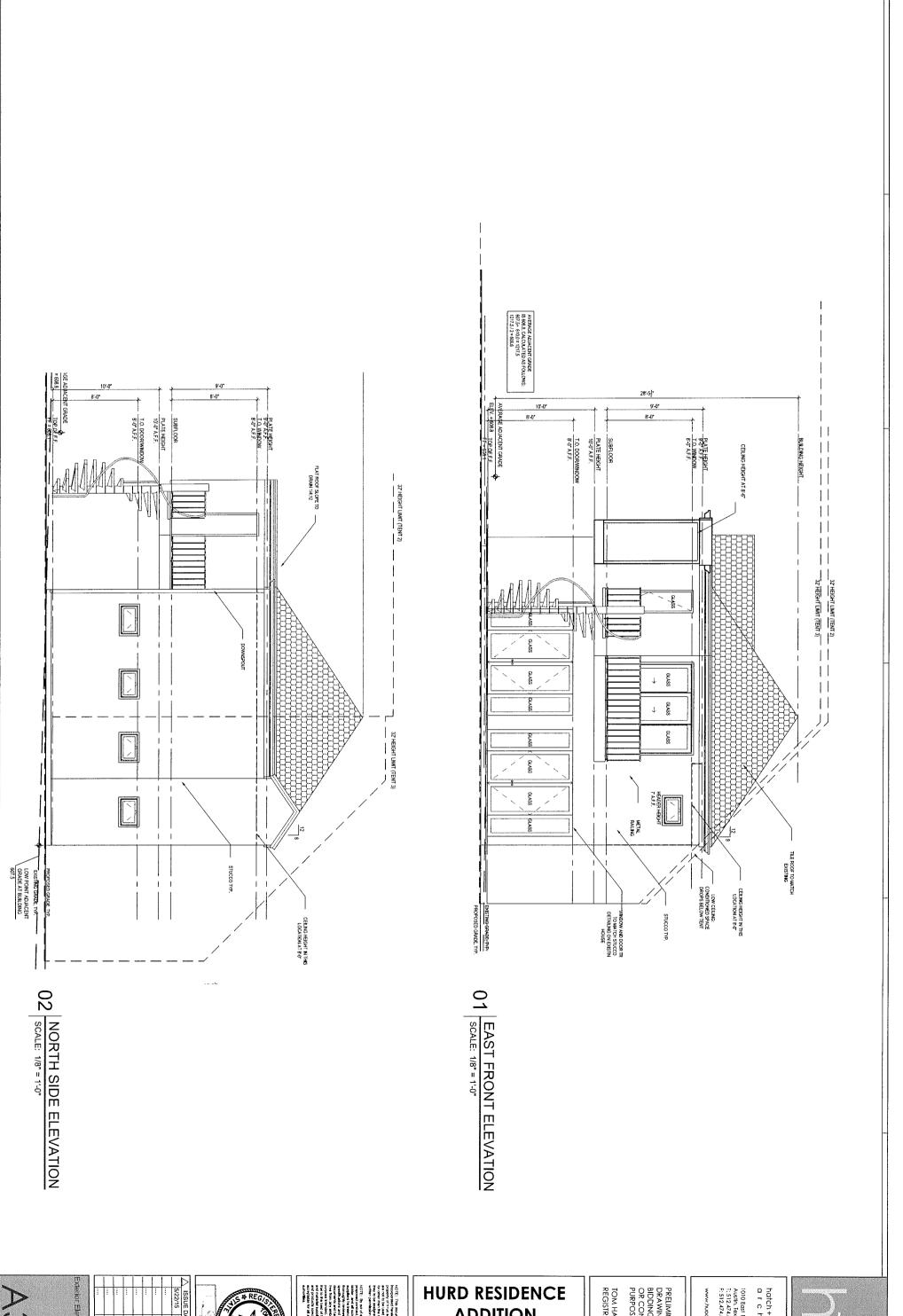
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### **ADDITION**

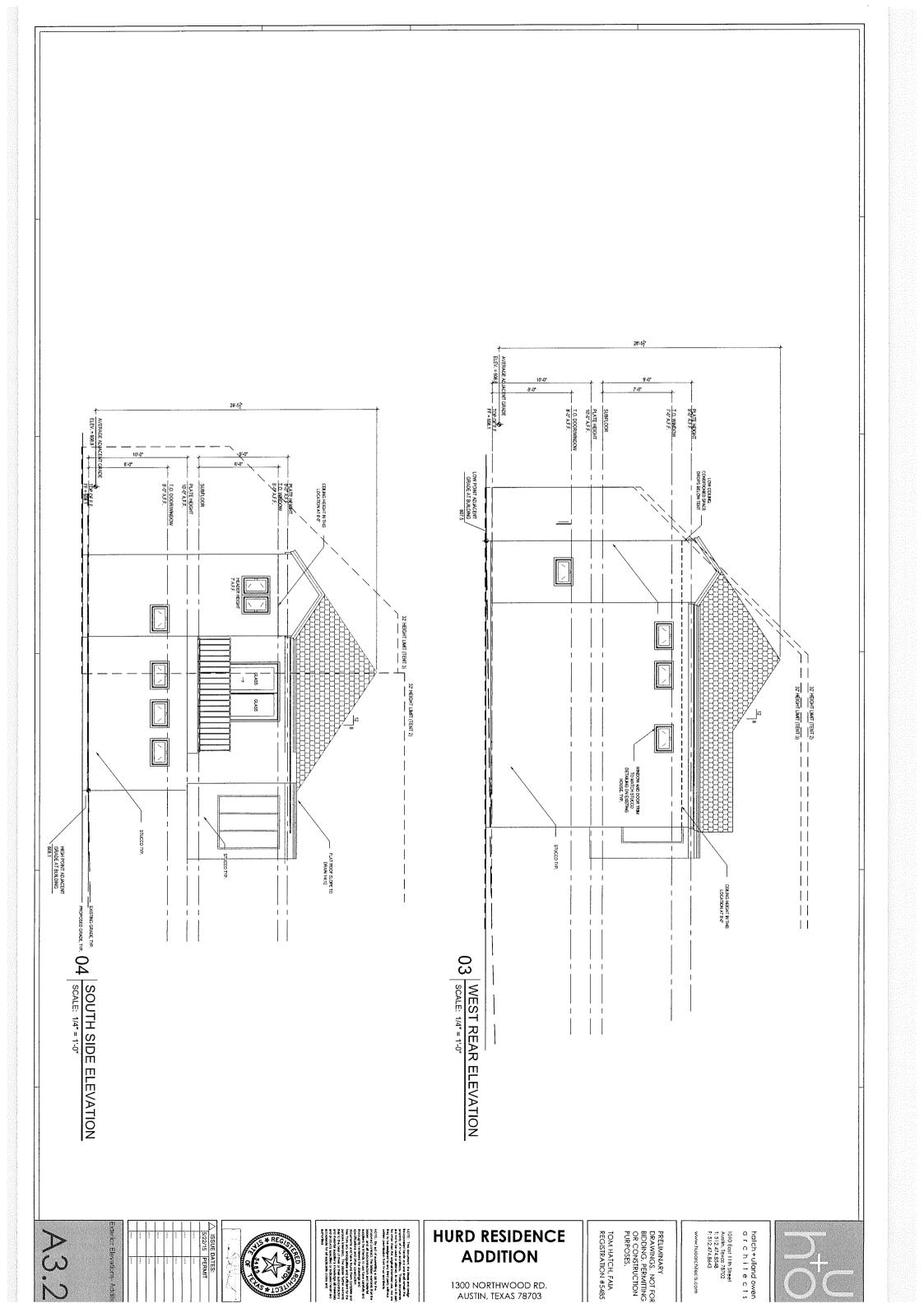
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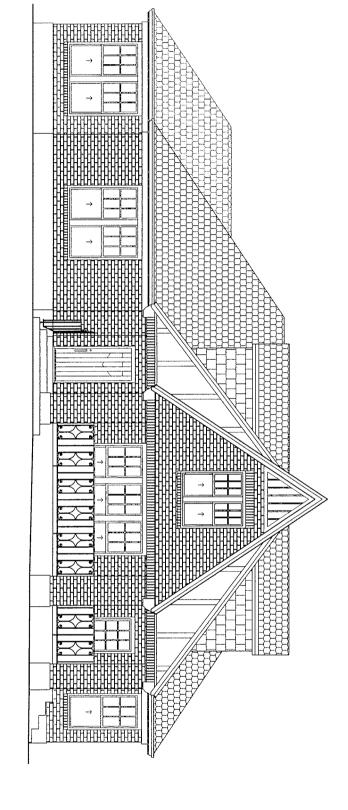
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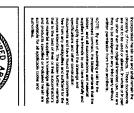


01 FRONT SOUTH ELEVATION
SCALE: 1/8"=1'-0"



02 EAST SIDE ELEVATION
SCALE: 1/8" = 1'-0"







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PRELIMINARY
DRAWINGS: NOT FOR
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OR CONSTRUCTION
PURPOSES.

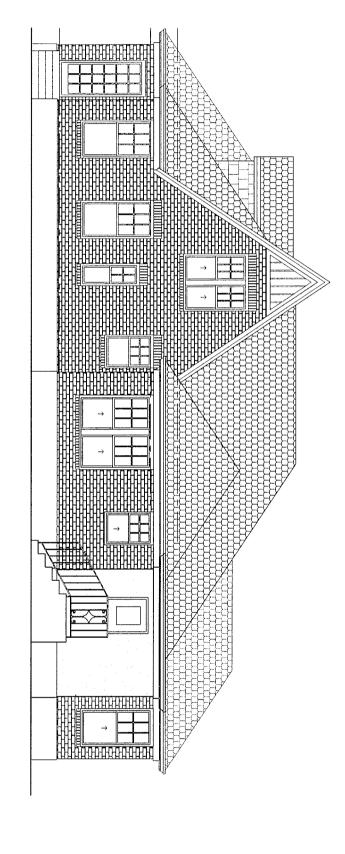


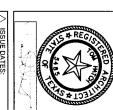
04 WEST SIDE ELEVATION

SCALE: 1/8" = 1'-0"



03 NORTH REAR ELEVATION
SCALE: 1/8" = 1'-0"





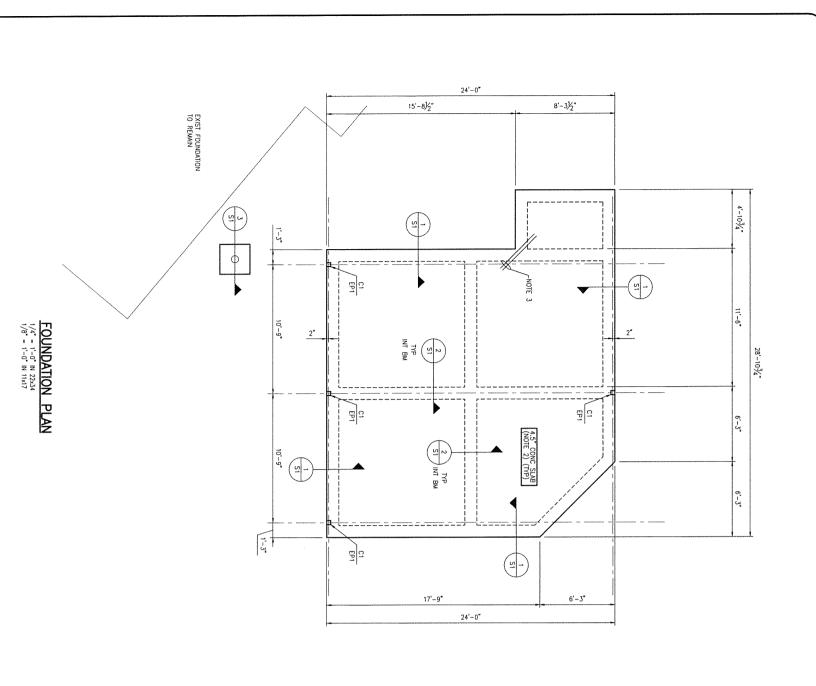




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- Prior to placing fill material, remove all organic and other deleterious material from the existing subgrade for a distance of Z-D beyond building line. All exposed surfaces shall then be recompacted to a minimum of 95 percent of the maximum dry deality as defined by 1900 test method TEX 113-E or 114-E at a moisture content within 3 percent of the aptimum moisture content.

- The use of fly ash is recommended, but shall not exceed 25% of the lotal of the cement plus fly ash by weight.
- CAST IN PLACE CONCRETE Cast in place concrete shall meet

28 Day Aggregate Class Strength Type Size A 3000 psi C 33 1

- Structural file shall be placed in 8 inch loade litts, watered as required and composited to a minimum 95 percent of the maximum dry density as defined in 7,000 test method TEX 113-E at a moisture content within 3 percent all the optimum moisture content.
- Provide a 10 mil polyaletin Stego retarder. Place vapor barrier in accordance with manufacturer's recommendation on top of structural fill.

2. 4.5" SLAB OVER VAPOR RETARDER OVER COMPACTED STRUCTURAL FILL REINF SLAB WITH \$3.0 "12" O.C.E.W. AT MID DEPTH, SEE NOTES FOR STRUCTURAL FILL REQUIREMENTS.

3. 2-\$3x4'-0" CORNER BARS - TYPICAL AT ALL RE-ENTRANT CORNERS.

4. C1 = HSS4x4x1/4 COLUMN. EP = EMBED PLATE - SEE DETAILS.

PLAN NOTES:

1. VEREY ALL DIMENSIONS, SLAB DROPS W/ ARCHITECTURAL DRAWNINGS PRIOR TO STARTING WORK.

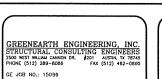
- Reinforcing steel shall be del
   Detailing of reinforcing steel s
- Provide Z-f4 bent bar with Z- $0^\circ$  legs top and bottom in interior and exterior face all grade beams at corners and top and bottom in exterior face of grade beam at intersections.
- Welding of reinforcing steel will not be permitted. Heat shall not be used in the fabrication or instal labrication or installation of reinforcement
- a. Grade beams 1 1/2" t shall be as follows: top, 3" bottom, 2" side (formed), 3" side (placed earth)

<u>S</u>

accordance with ASTM A615 Grade 60.

shall conform to the American. Concrete Institute Detailing Manual.

HURD RESIDENCE 1300 NORTHWOOD ROAD





Zhigang Zhang 06/29/2015

DETAILS

SCALE

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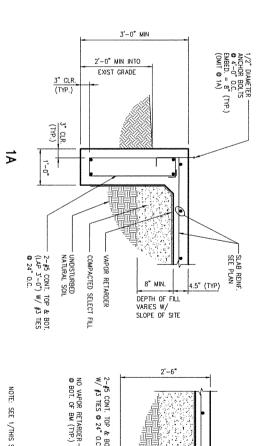
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"=1'-0'

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11X17

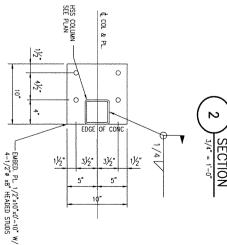
SHEET



CONT. TOP & BOT.-3 TIES @ 24" O.C.

NOTE: SEE 1/THIS SHEET FOR INFORMATION NOT SHOWN

SECTION 3/4" = 1-0"



1'-6" MIN INTO

EXIST GRADE

BASE PLATE BY MFR COLUMN : BY MFR

3-#4 EA WAY ---

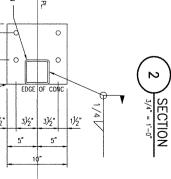
SECTION 3/4" = 1'-0"

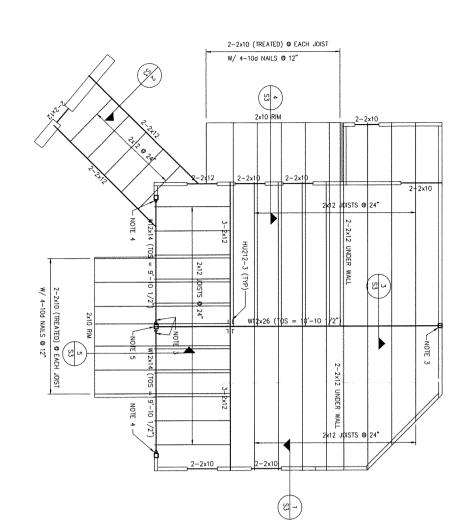
EMBEDDED PLATE DETAIL

EP1

FOOTING SHALL -BEAR ON ROCK

-6" (SQUARE)





# 2ND FLOOR FRAMING PLAN 1/4" = 1"-0" IN 22x34 1/6" = 1"-0" IN 11x17

PLAN NOTES:

1. BEAMS AND HEADERS SHALL BE SUPPORTED BY BUILT-UP COLUMNS & BLOCKED SOLID TO FOUNDATION. SEE THIS SHEET FOR BUILT-UP COLUMN SIZES.

2. ALL WALLS SHOWN ARE 2x4 @ 16" O.C. LOAD BEARING WALL U.N.O. IN THIS PLAN. SEE ARCH'L FOR 2x6 PLUMBING WALLS.

4. SEE 7/S3 FOR STEEL MOMENT CONNECTION DETAIL

3. 2X6 CEILING JOISTS @ 24" O.C. 2. BEAMS AND HEADERS SHALL BE SUPPO TO FOUNDATION. SEE SCHEDULE IN THIS

RTED BY BUILT-UP COLUMNS & BLOCK SOLID SHEET FOR BUILT-UP COLUMN SIZES.

4. O DESIGNATES 2-2x4 BRACE TO CEILING BEAM BELOW.

PLAN NOTES:

1. ALL EXT. WALLS ARE 2x4 @ 16" LOAD BEARING WALLS UNLESS NOTED OTHERWISE

ALL INTERIOR WALLS ARE 2x4 @ 16" LOAD BEARING WALLS UNLESS NOTED OTHERWISE
IN THIS PLAN.

SEE ARCH'L FOR 2x6 PLUMBING WALLS.

ROOF FRAMING

1/4" = 1'-0" IN 22x34

1/8" = 1'-0" IN 11x17

PLAN

3. SEE 6/S3 FOR STEEL BEAM OVER HSS COLUMN CONNECTION DETAIL.

מטובו-טד נט	BUILI-UF CULUMN SCHEDULE
BEAM SIZE	BUILT-UP STUD COLUMN
2- 1 3/4"x14" LVL & UP	4- STUD COLUMN
2- 1 3/4"x12" LVL	3- STUD COLUMN
3- 2 x	3- STUD COLUMN
2-2 × 12	3- STUD COLUMN
2- 2 x 10 OR SMALLER 2- STUD COLUMN	2- STUD COLUMN

## RCH CEILING 2-2×8 2x8 RAFTERS @ 24" O.C. - TYP

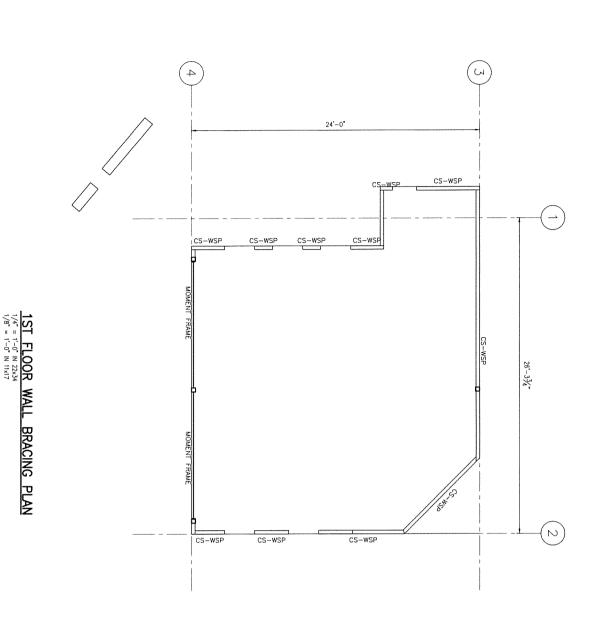
Zhigang Zhang 06/29/2015

S2.1

HURD RESIDENCE

GREENEARTH ENGINEERING, INC.
STRUCTURAL CONSULTING ENGINEERS
2500 WEST WILLIM CANNON DR. #201 ALSHIN, 17 ASP
PHONE (512) 289-8086 FAX (512) 462-0800





LEGENDS: BML = BRACED WALL LINE; CS-WSP = CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL;
GB = GYPSUM BOARD: CS-PF = CONTINUOUS SHEATHED PORTAL FRAME. 1 OF 2 1 OF 2 2 OF 2 1 OF 2 BWL SPACING (FEET) BRACE WALL LINE DATA REQUIRED LENGTH (FT) MOMENT FRAME 8+ 8+ PROVIDED LENGTH (FT) 12+ 12+

1. BRACED WALL LINE DATA BASED ON SEISMIC DESIGN CATAGORY A AND A WIND SPEED OF 90 MPH OR LESS.
2. MAXIMUM BWL SPACING SHALL NOT EXCEED 60 FEET O.C.;

2ND FLOOR WALL BRACING
1/4" = 1"-0" IN 22:24
1/8" = 1"-0" IN 11:17

PLAN

- 3. WOOD STRUCTURAL PANEL SHALL BE  $1/2^\circ$  OSB (OR PLYWOOD) SHEATHING AND SHALL BE FASTENED WITH 8d COMMON NAILS  $\otimes$   $6^\circ$  O.C. AT PANEL EDGES AND  $12^\circ$  O.C. AT INTERMEDIATE FRAMING.
- 4. ALL HORIZONTAL PANEL SPLICES SHALL BE BLOCKED WITH BLOCKING EQUAL TO WALL STUD SIZE AND SHALL BE FASTENED WITH Bd COMMON NAILS @  $6^{\circ}$  O.C.
- THE FLOOR DECK SHALL BE 3/4" MIN OSB OR PLYWOOD DECK AND SHALL BE FASTENED WITH BIJ COMMON NAILS. © 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE FRAMING.
- 6. THE ROOF DECK SHALL BE  $5/8^{\circ}$  Min OSB or plywood deck and shall be fastened with Bd common nails  $\oplus$   $6^{\circ}$  O.C. at panel edges and 12 $^{\circ}$  O.C. at intermediate framing.
- 7. GYBSUM BOARD SHALL BE 1/2" THICK AND SHALL BE FASTENED WITH 6d COMMON NAILS  $\oplus$  6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE FRAMING.

26'-11/4" (b)

> Zhigang Zhang 06/29/2015

HURD RESIDENCE 1300 NORTHWOOD ROAD GREENEARTH ENGINEERING, INC.
STRUCTURAL CONSULTING ENGINEERS
2500 WEST WALLAN CANNON DR. #201 AUSTIN, TX 78745
PHONE (512) 289-8086 FAX (512) 462-0800 GE JOB NO.: 15096



## STRUCTURAL GENERAL NOTES

DETAILS

SCALE

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/8"=1

0,

 $\equiv$ 

11X17

SHEET

- Building Code: 2012 Inter
- Wood Framing: National Design Specifications For Wood Construction with Supplement, National Forest and Paper Products Association, Latest Edition.
- Prefabricated Metal Plate Connected Wood Trusses: Design Standard for Metal Plate Connected Wood Truss Construction, ANS/TPI 1-95. Structural Plywood: Plywood Design Specification, American Plywood Association, Latest Edition.
- DESIGN LOADS

### 1. Live Loads

a. Roof b. Floor c. Stair

## TIMBER FRAMING

- Unless otherwise noted, all structural framing lumber shall be chearly marked no. 2 southern yellow pine or dougles fix, except that non-loadbearing interior walls may be stud grade southern yellow pine, dougles fix, or spruce-pine-fix.
- All wood headers, beams, and top plates shall be no. 2 Southern Yellow Pine or Douglas Fir.
- All load bearing walls shall have solid 2x blacking at 4"-0" a.c. maximum vertically. End noil with 2-16d noils or side toe noil with 2-16d noils. . Provide double studs at all wall corners and on each side of all openings, unless noted or detailed otherwise.
- The entire exterior wall framing shall be braced by a 1/2" thick pand of JAA rated sheathing with an exposure 1 rating extending from the top plate to the sill pales. Where wall is alter than 6"-0" provide ruitigle panels as required to extend from sill plate to top plate. Provide 2s blocking as required to support oil panel edges, this with 6d common noils of 6" on center of supported edges and 12" on center of intermediate supports.
- Solid 2x blocking or bandboard shall be provided at supports and confliever ends of all wood pists, and between supports in rows not exceeding  $\theta^*-\theta^*$  apart.
- All framing members framing into the side of a header, hip, valley, idage, truss or any other beams shall be elloched using melo just hangers manufactured by the Simpson Company or equal. The hanger shall be sized and installed in accordance with the manufacturers recommendations for the size of joist supported.
- Proce a single plate at the bottom and a double plate at the top of all stud walls. Exterior sill plates shall be batted to the foundation with  $1/2^n$  anchor botts with a minimum embedment of  $6^n$  spaced at  $4^n$ - $0^n$  on center. Provide a minimum of we botts per plate segment. Sill plates in contact with concrete or masonry shall be pressure treated with a preservative. Neiling and attechment of all framing numbers and sheathing shall be as specified in the International Residential Code Heiling Scheduler (Fable R062.7) unless noted otherwise in the Orening. Common wire nails or spikes, or gotherized box nails shall be used for all framing unless noted otherwise.
- 10. Provide double joists under all interior partition walls oriented parallel to the joists.
- Provide triple studs (or triples) at each end of any header, beam, ridge, valley, or his spanning over  $10^{-0^{\circ}}$  unless noted otherwise. Provide double studs (or cripples) at each end of any header, beam, ridge, valley, or his spanning 5°-0° to  $10^{\circ}$ 0° unless noted otherwise.
- The new generation of pressure treated humber products are highly corrosive to metal consists and destenders.

  All tasterers and metal connectors used in conjunction with the new generation of pressure treated where she half-op galvanized or stainless steel. These locations include, but are not limited to the following:

- FLOOR JOIST SEE PLAN

- Archor botts at sole plate to foundation.
  Nails form adia plate to wall stude.
  Nails form adia plate to wall stude.
  Nails at exterior playmood sheathing to sole plate.
  Botts at ledger to concrete.
  Licit to treated ledger connections.
  All hangers on treated joists.
  Wood posts to concrete.
  Wood posts to concrete.

# PREFABRICATED METAL PLATE CONNECTED WOOD TRUSSES

- Truss members shall be clamped in a mechanical or hydraulic fig with sufficient pressure to bring members into reasonable contact at all joints during application of connector plates Trusses shall be designed by the Contractor in accordance with the Truss Plate Institute "Design Standard for Wetal Plate Connected Wood Truss Construction" (ANS/TP1 1—95).
- 3. Provide adequate erection bracing in accordance with Truss Plate Institute publication HiB-91. Truss Manufacturer shall provide permanent bracing as required by the design of the trusses. Exection bracing may remain in place as permanent bracing where it does not interfere with the architectural finishes.
- All timber truss members shall be Southern Yellow Pine with a maximum moisture content of 19%. Chard members shall be no. 2 or better and web members shall be no. 3 or better.
- Connection plates shall be manufactured by a WTCA member plate manufacturer. Plates shall be 20 gauge minimum, ASTM A446 grade A steel, with a 660 galvanized coaling.
- Trusses shall be designed in accordance with the following requirements:

5

SECTION 3/4" = 1'-0"

တ

TYPICAL DETAIL
NOT TO SCALE

TYPICAL DETAIL
NOT TO SCALE

SIMPSON LSSU JOIST HANGER

2x4 SILL PLATE W/ 1/2" DIAMETER
ANCHOR BOLTS
Ø 4'-0" O.C.
EMBED. = 8" (TYP.)

SEE PLAN

BALCONY JOIST SEE PLAN 1-10d NAILS @ 12"

-STL COL. SEE PLAN

-1/4"x4"x4" COLUMN CAP

Top charts shall be designed to resist the local bending induced by the floor or roof uniform load on the top chard.

- Limit live load deflection of floor trusses to L/480. Total load deflections shall be limited to L/360.

## COMPOSITE WOOD MEMBERS

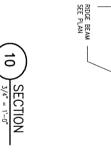
- Where noted on the drawings, joists shall be TJ "SP" series engineered wood joists, and beams shall be "Micro-Lam" or "Paraliam" beams as manufactured by the Trus Joist Macmilian Corporation.
- Do not notch joists or beams. Drill holes through webs of engineered wood members for mechanical, electrical or plumbing services in accordance with the recommendations of the engineered wood product manufacturer.
- Multiple wood beams up to three members thick shall be noised together with three rows of 16d nois at  $12^{\circ}$  or center. Four or more multiple wood beams and any multiple wood beams utilizing beams thicker than  $1.3/4^{\circ}$  shall be boiled depther with  $1/2^{\circ}$  denoter boils top and bottom of supports and ends of the beam, then at  $24^{\circ}$  on center, staggered top and bottom for the full length of the beam.
- Where multiples of two 1  $3/4^{\circ}$  Micro-Lam beams are noted on the drawings, contractor may provide single 3  $1/2^{\circ}$  beams in fleu of double 1  $3/4^{\circ}$  beams.
- Provide web slifteners where required by the manufacturer for the specified support condition.

- Structural Steel M-sängbre pinde conform tol M592. Steel plates, angles, and channels may be ASTA 1872, 1904 5.0 or ASTA M-sängbre plate soll conform to ASTA 550; fige E or S, grade B. Steel tube shall conform to ASTA 5500, grade B, Fy 46 ksi.
- Column base plates shall be grouted with a nonshrink, high strength nonmetallic grout. Pre-grouting of column base plates will NDT be permitted.
- Splicing of structural steel members is prohibited without prior approval of the Engineer as to location and type of splice to be made. Any member having splice not shown and detailed on shop drawings will be rejected.

## STRUCTURAL STEEL CONNECTIONS

- Balls shall conform to ASTM A325. Balls shall be designed using values for bearing type balts with thread allowed in the shear plane. Welding shall conform to ANSI/AWS D1.1, latest edition

### -SEE ARCH FOR RAFTER TAIL DETAIL (TYP) SIMPSON H2.5 HURRICANE ANCHOR C 4-0 O.C. rafter see plan 5/8" PLYWOOD/OSB —— ROOF DECK W/ RADIANT BARRIER (TYP.)





TYPICAL COMECTION DETAIL BETWEEN EXTERIOR WALL & FOUNDATION

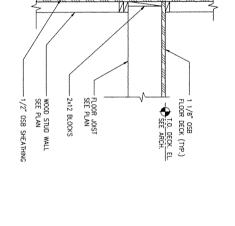
SECTION

3/4" = 1'-0"

**S3** 

9







-WOOD BEAM

SEE PLAN

2x12 LEDGER W/ ----2-1/2\*# THRU BOLTS © 24\* O.C.

-STL BEAM SEE PLAN

-2x10 RIM W/ 4-10d END NAILS INTO EA JOIST

-BALCONY JOIST SEE PLAN

·10d NAILS @ 12"

JOIST SEE PLAN

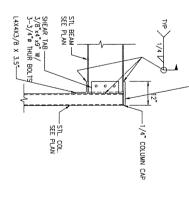
JOIST SEE PLAN

-- 2x4 NAILER W/ HILTI X-DNI 47 PB FASTENERS @ 6\* STAGGERED

JOIST HANGER

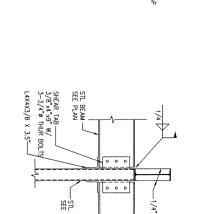


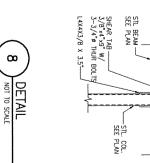




-1/4" STIFFENER BOTH SIDES











FLOOR JOIST SEE PLAN

DECK - SEE ARCH RAILING - SEE ARCH



Zhigang Zhang 06/29/2015

1300 NORTHWOOD ROAD





















