

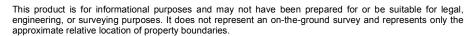




PENDING CASE

ZONING BOUNDARY

CASE#: C16-2018-0003 LOCATION: 1044 Norwood Park Boulevard





B01/2

Board of Adjustment Sign Variance Application

WARNING: Filing of this appeal stops all affected construction activity.

This application is a fillable PDF that can be completed electronically. To ensure your information is saved, click here to Save the form to your computer, then open your copy and continue.

The Tab key may be used to navigate to each field; Shift + Tab moves to the previous field. The Enter key activates links, emails, and buttons. Use the Up & Down Arrow keys to scroll through drop-down lists and check boxes, and hit Enter to make a selection.

The application must be complete and accurate prior to submittal. All information is required (if applicable).

For Office Use Only	100 2116/2/1
Case #C16-2018-00 ROW# 11941413	Tax#
Section 1: Applicant Statement	
Street Address: 1044 Norwood Park Blvd.	
Subdivision Legal Description:	
LOT 5 LESS 2464 AC WAL-MART AT NORWOOD PARK SU 1C & LOT 2 REPLAT OF NORWOOD PARK	BD RESUB OF LOTS 1A,1B &
Lot(s): Block(s):	
Outlot:	
Sign District:	
I/We Phil Moncada	on behalf of myself/ourselves as
authorized agent for Norwood Park Association, Inc	affirm that on
Month April , Day 25 , Year 2018 , he	reby apply for a hearing before the
Board of Adjustment for consideration to (select appropriate opt	ion below):
○ Erect ○ Attach ○ Complete ○ Remodel ○ Mai	ntain Other: relocate/height increase
Type of Sign: pylon	
Portion of the City of Austin Land Development Code applicant is s	eeking a variance from:
City of Austin Board of Adjustment Sign Variance Application	09/11/2015 Page 2 of 4

B01/3

Section 2: Variance Findings

The Board must determine the existence of, sufficiency of, and weight of evidence supporting the findings described below. In order to grant your request for a variance, the Board must first make one or more of the findings described under 1, 2, and 3 below; the Board must then make the finding described in item 4 below. If the Board cannot make the required findings, it cannot approve a sign variance.

Therefore, you must complete each of the applicable Findings Statements as part of your application. Failure to do so may result in your application being rejected as incomplete. Please attach any additional supporting documents.

I contend that my entitlement to the requested variance is based on the following findings:

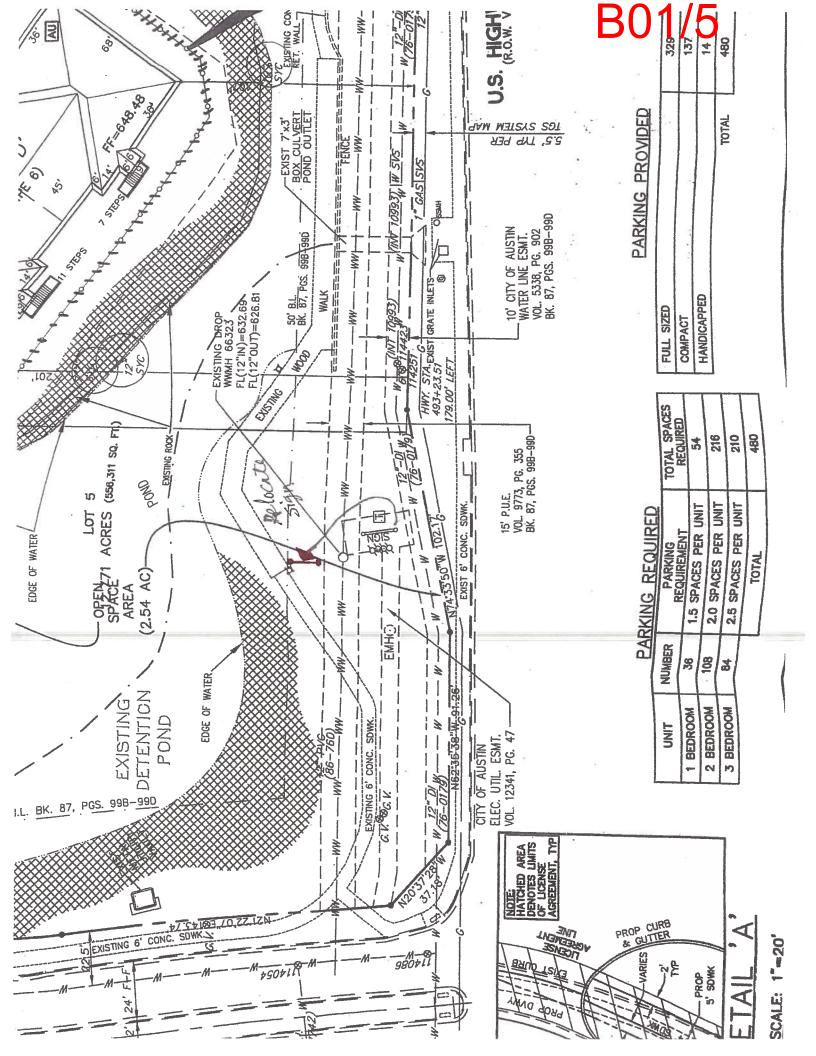
officing that my childerness to the requested variance is based on the	ie following littuings.
 The variance is necessary because strict enforcement of the A opportunity to provide adequate signs on the site, considering such as dimensions, landscaping, or topography, because: 	
TXDOT_ROW_Condemnation process has already removed s In addition, existing trees and speed limit an access road, hin additional height is granted.	
OR	
The granting of this variance will not have a substantially adve properties, because:	erse impact upon neighboring
Sign is on access and surrounded by commercial properties.	
 —OR— 3. The granting of this variance will not substantially conflict with ordinance, because: 	the stated purposes of this sign
Sign was existing at this location and height increase is warra associated with access road.	anted due to line and sight
AND, 4. Granting a variance would not provide the applicant with a sp others similarly situated or potentially similarly situated, becau	
This board has previously granted height increase on signs visibility for the motoring public.	associated with trees impacting
of Austin I Poord of Adjustment Sign Variance Application	09/11/2015 I Page 3 of 4

B01/4

Section 3: Applicant Certificate

my knowledge and belief.	The complete application are the and	correct to the best of
Applicant Signature: Plice Monce	Digitally signed by Phil Moncada Date: 2018.04 10:35:15 -05'00'	Date: 04/19/20 18
Applicant Name (typed or printed): Phil		100 mas 100 ma
Applicant Mailing Address: 1301 S IH 3	35, Ste 204	
City: Austin	State: TX	Zip: 78741
Phone (will be public information): (512	2) 627-8815	
Email (optional – will be public informat	ion):	
Section 4. Owner Cortificate		
Section 4: Owner Certificate		
affirm that my statements contained in my knowledge and belief.	the complete application are true and	d correct to the best of
ny knowledge and belief.	enke, Bouardmember	464/10
-//		Date: 1/201/8
Owner Name (typed or prifited): <u>Norwo</u>	od Park Association, Inc.	
Owner Mailing Address: PO Box 16115	50	***************************************
City: Austin	State: TX	Zip: <u>78716</u>
Phone (will be public information): (512	2) 485-4334	
Email (optional – will be public informat	tion):	
Santian = Agant Information	in the	
Section 5: Agent Information	711	
Agent Name: <u>Greg Cervenka</u>		
Agent Mailing Address: PO BOX 1611	150	
City: Austin	State: TX	Zip: 78716
Phone (will be public information): <u>(51</u>	2) 485-4335	
Email (optional – will be public informat	tion):	

SAVE





April 19, 2018

Structural Calculations

Prepared For:

Facility Solutions Group 10212 Metric Blvd. Austin, TX. 78758

Project

JTS_74218
Norwood Assn — Pylon A
1030 Norwood Park Blvd.
Austin, TX

Prepared By:

YJ Inc. P.O. Box 802050 Santa Chrita, CA 91380



Total 4 - pages including cover

-12'-0" 141-71

Project Job Location

Norwood Assn - Pylon A 1030 Norwood Park Blvd.

JTS_74218

Austin, TX

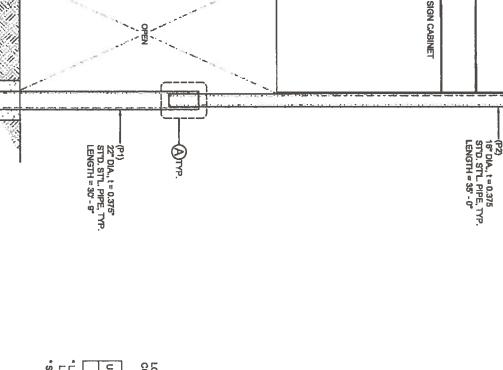
Sign Design Based on 2015 IBC

16'-6 1/2"



73.0° 4 48" Ø ELEVATION

—CONSOLIDATED CONC. f'c≈2500 PSI TYP.



SEE TABLE

2-3 (TYP.)

RING PLATE SNUG FIT TO I.D. OF PIPE

evaluated at height above ground level, h. (Teb. 29.3-1, pg 310) K_d = wind directionality factor. (Teb. 26.6-1, page 250)

11 Ħ

0.85 1.09

q_h = velocity pressure at height h. (Eq. 29.3-1, page 307)

¬ velocity pressure exposure coefficient

9 = 0.00255 K, K, K, K

31.37 psf

lelocity pressure

Dimension of return comer

lorizontal dimension

14.25 ft

영

멸

Aertical dimension (for wall, s = h)

Height of the sign

opographic factor

Basic wind speed (3 sec. gust wind)

disk Category

Exposure category (B, C or D)

INPUT DATA

人。 TABLE

SEE TABLE

1 4

RING PLATE SEE TABLE

NOTES:

GENERAL:

PROVIDE ISOLATION OF DISSIMILAR MATERIALS. SIGN DESIGN IS BASED ON ADEQUATE EXISTING SUPPORT ELEMENTS.

COAT ALUMINUM IN CONTACT WITH CONCRETE WITH ZINC RICH PAINT. THERE IS NO PROTECTION ZONE AS DEFINED IN AISC 341-10.

PROVIDE FULLY WELDED END CAPS AT EXPOSED OPEN ENDS OF STEEL / ALUM. TUBES, MATCH THICKNESS LIKE FOR LIKE.

CABINETS SHALL BE CONSTRUCTED OF NONCOMBUSTIBLE

ANCHORS: SLOPE TOP OF EXPOSED FOOTING AWAY FROM DIRECT BURIAL POSTS BRAND NAME APPROVED POST INSTALLED ANCHORS SPECIFIED ON PLANS MAY BE SUBSTITUTED BY APPROVED EQUAL.

ALL STEEL MACHINED BOLTS SHOULD BE: ASTM F1554
ALL STEEL MACHINED BOLTS SHOULD BE: ASTM A307
ALL STAINLESS STEEL MACHINED BOLTS SHOULD BE: ASTM F583
ZINC COATED (HOT DIPPED) PER: ASTM A163 OR F2329
BEARING TYPE CONNECTION REINFORCING REBAR: ASTM A615 GRADE

80 DEFORMED BARS

DESIGN AND FABRICATION ACCORDING TO 2015 IBC

PLATE, ANGLE, CHANNEL TEE, AND WIDE FLANGE: ASTM A38

ROUND PIPE: ASTM A33 GRADE B OR EQUIVALENT.

HSS ROUND, SQUARE, AND RECTANGULAR TUBE: ASTM A500 GRADE B OR EQUIVALENT

DESIGN AND FABRICATION ACCORDING TO 2015 ALUM. DESIGN MANUAL
PILATES, ANGLES, CHANNELS, TEE AND SQUARE TUBING: ALUMINUM
ALLOY 6061 - TS WITH 0.086 LBS PER CUBIC INCH. MUNIMU

WELDING:

STEEL DESIGN AND FABRICATION ACCORDING TO AWS D1.1.

AWS CERTIFICATION REQUIRED FOR ALL STRUCTURAL WELDERS.

WELDING PER AISC 341-10

FOR XX ELECTRODE FOR SMAW PROCESS.

F70 XX ELECTRODE FOR GMAW PROCESS.

ER7 XX ELECTRODE FOR GTAW PROCESS.

ER7 XX ELECTRODE FOR FCAW PROCESS.

ALL WELDS SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY VANOTCH TOUGHNESS OF 20FT-LB AT ZERO 0° AS DETERMINED BY THE APPROPRIATE AWS AS CLASSIFICATION TEST METHOD OR MFG'S, CERTIFICATION.

ALL WELDING IN ACCORDANCE WITH THE LATEST EDITION OF THE AWS A.5.10. FILLER ALLOYS PER TABLES M.B.1 & M.9.2 OF 2015 ALUMINUM DESIGN MANUAL.

CONCRETE:

DESIGN AND CONSTRUCTION ACCORDING TO ACI 318-14
- COMPRESSIVE STRENGTH AT 28 DAYS, 1'c=2500 PSI

CEMENT TYPE II OR IV. WIC RATIO 0.45 BY WEIGHT FOR MINIMUM.

MAINTAIN A MINIMUM 3" CONCRETE COVER OVER ALL EMBEDDED STEEL. CONCRETE MUST BE POURED AGAINST UNDISTURBED

LATERAL SOIL BEARING PER IBC CLASS & TABLE 1806.2 (100 PSF/FT).

plotted by: yinc on 4.18.2018 @ 2:00 PM PROJ. START DATE: SCALE: Apr 18, 2018 Apr 17, 2018 AS SHOWN REV. NO. REVISED BY PROJECT LOCATION: PROJECT JOB #: JTS_74218_Norwood Assn_Pylons_Norwood Park Bivd_Austin_TX.dwg NORWOOD ASSN 1030 NORWOOD PARK BLVD. AUSTIN, TX SHEET# 유

MORTE A MOSTE REVBY: DRN BY: A.W. CHK BY: R.T.

P.O. BOX 802050 SANTA CLARITA, CA. 91380 TEL. (661)259-0700 FAX. (881)259-0900 www.yjinc.com

SHEET TITLE:

COLUMN SPECIAL INSPECTION REQUIRED FOR FIELD WELD UPPER COL. DIA. LENGTH OF PLUG WELDS TO BE 1/8 OF LOWER COLUMN DIA., MINIMUM 1/2" 16" Ø 4-1 A STEP DOWN ą 36 WELD SIZE 1/4" FUIL PEN, PLUG WELD A PLACES @ 90° (TYP.) EA, RING PLATE RING PLATE SNUG FIT TO I.D. OF PIPE RING PL 12
 Pole (P2) Design
 Std. Steel Pipe

 Sec. Mod. Req'd.
 USE JA53 Grade B

 S = 154.67
 18" Dia., t=0.376
 S=66.67
 Wind Force Case A: resultant torce though the geometric center (Sec. 29.4.1 & Fig. 29.4-1)

Max horizontal wind pressure = p = q_h GC₇ = 48.11 psf

where G = gust offect factor. (Sec. 28.9, page 254). = 0.88 DESIGN SUMMARY Sec. Mod. Radd. Pole (P1) Design Footing Design | See attached Enercalc calcs | Unfactored Windforte, F = 48.11 x As = 110,52 Design Moment = F x moment arm = Design Windforce, F = Unfactored Moment = F x moment arm = A = B s = the gross area C,= net force coefficient. (Fig. 29.4-1, page 308) Allowable Stress Design Wind Factor = 0.6

USE A53 Grade B 22" Dia., t=0.375 S=126.40

Srd. Steel Pipe

0.6 x p = 27.67 x As = Moment Arm =

27.67 psf 11.77 kips 32.86 ft 386.8 kip-ft

13'-9"

Estimated sign weight =

4265 Lbs 426.5 n²

1.73

19.62 klps 644.7 ktp-ft

YJ INC.

1/8

Norwood Assn J.J. Pylon Concrete Footing

File = ZYVJSIGN-32018JT-1174218_*1JTS_74-1 EC6 ENERCALC, INC 1983-2017, Build: 10.17.8.29, Ver 10.17.8.29 E1CET15.00 [V2] [NO.]

Project Title: Engineer: Project Descr.

Project ID: JTS_74218

Pole Footing Embedded in Soil 能管理系KW和6600918Z Description: Pylon A Concrete Footing

Code References

Calculations per IBC 2015 1807.3, CBC 2016, ASCE 7-10 Load Combinations Used: IBC 2015

General Information

200.0 pcf 1,500.0 psf Circular 48.0 in

Controlling Values Moment Governing Load Combination: +D+0.60W Pressures at 1/3 Depth Lateral Load Allowable Actual NO Ground Surface Restraint 5.886 k 193.414 k-ft 910.90 psf 911.90 psf

Applied Loads

Footing Base Area Meximum Soli Pressure

12.566 ft^2 0.1016 lesf

Minimum Required Depth

13.750 ft

ateral Concentrated Load (k)	3	Lateral Distributed Loads (kit)
D : Dead Load	~	K/III
Lr: Roof Live	*	C/fi
L:Live	*	
S:Snow	×	
W: Wind	9.810 k	K/fi
E : Earthquake	~	
H: Lateral Earth	*	
Load distance above		TOP of Load above ground surface
ground surface	32.860 ft	⇒
		BOTTOM of Load above ground surface

Load Combination Results

	Forces @	Ground Surface	Required	Pressure at	1/3 Depth	Solling
Load Combination	Loads - (k)	Mornents - (ff-k)	Depth - (ft)	Actual - (psf)		Fact
D Only	0.000	0,000	0.13	0.0		
+D+0.60W	5,886	193,414	13.75	910.9		<u> </u>
+D-0.60W	5.886	193.414	13.75	910,9		<u> </u>
+D+0.450W	4.415	145.060	12,38	820.4		<u> </u>
+D-0.450W	4.415	145.060	12.38	820.4		
+0.60D+0.60W	5.886	193.414	13.75	910.9		
+0.60D-0.60W	5,886	193.414	13.75	910.9		1.0
+0.60D	0.000	0.000	0.13	0.0	0.0	1.000

YJ INC.

B01/9

Project Title: Engineer: Project Descr. Norwood Assn J.J. Pylon Concrete Footing

Project ID: JTS_74218

Pole Footing Embedded in Soil គ្រាំង ្រូវ (អ្វី ស្រួល នៃ ក្រុង ខេត្ត Description : Pylon B Concrete Footing Printed 18 APR 2018, 8:24AM File = Z/YL/SIGN-32018JT-1\74218_-1\JTS_74-1 EC6 ENERCALC, INC 1983-2017 Build:10 17 8.29 Ver 10.17 8.29 ETG-97/Sign-2-70/1NG.

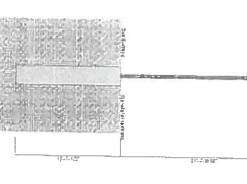
Calculations per IBC 2015 1807.3, CBC 2016, ASCE 7-10 Load Combinations Used: IBC 2015 Code References

General Information

Circular 30.0 in

200.0 pcf 1,500.0 psf

Controlling Values Governing Load Combination: +D+0.60W Lateral Load Allowable NO Ground Surface Restraint 4.806 k 103.954 k-ft 905.66 psf 905.76 psf



Footing Base Area Maximum Soll Pressure Pressures at 1/3 Depth Actual Moment Minimum Required Depth 4.909 ft^2 0.2375 ksf 13.625 ft

Lateral Concentrated Load (k)	æ	Lateral Distributed Loads (kif)	Vertical Load (k)
D ; Dead Load	*		1 488 4
Lr: Roof Live	~		
L:Live	_		*C 2
Wous: S	~	Offe	ক ৫
W: Wind	8.010 k	k/ft	* 2
E : Earthquake	ж		₹ ;
H: Lateral Earth	<u></u>	K/ft	* 2
Load distance above		TOP of Load above ground surface	7
ground surface	21.630 ft	ROTTOM of load above ground surface	
		· ·	

	Forces @	Forces @ Ground Surface	Required	Pressure at 1/3 Depti	1/3 Depth	Soll Increase
Load Combination	Loads - (k)	Moments - (fi-k)	Depth - (ft)	Actual - (psf)	Allow (psil)	Factor
D Only	0.000	0.000	0.13	0.0	0.0	1.000
+D+0.60W	4.806	103.954	13.63	905.7	905.8	1.000
+D-0.60W	4.806	103.954	13.63	905.7	905,8	1.000
+D+0.450W	3.605	77.965	12.25	812.2	812.6	1.000
+D-0.450W	3.605	77.965	12.25	812.2	812.6	1.000
+0.60D+0.60W	4.806	103,954	13,63	905.7	905.8	1.000
+0.60D-0.60W	4.806	103.954	13.63	905.7	905,8	1.000
+0.60D	0,000	0.000	0.13	0,0	0.0	1.000