# **City of Austin - Bridges and Culverts**



### **Critical Infrastructure Assessment 2017**

# **Bridges**





#### Introduction

The approximate replacement value of Austin's bridge inventory of 2.25 Million square feet of bridge deck is about \$900 Million. The average Sufficiency Rating (SR) for Austin's 427 rated bridges (spans greater than 20 feet) is 83 (Very Good) on a scale from 0-100. This represents the nationally established criteria for the National Bridge Inspection System (NBIS) for rating and reporting bridge condition information to the Federal government. In general, Austin's bridges are in very good condition and mostly require only routine repairs and preventative maintenance.

The Sufficiency Rating (SR) as defined by National Bridge Inspection System (NBIS) is not just a simple measure of bridge safety only. These ratings include a variety of other criteria such as not meeting current standards for deck width, railing types, approach design, etc. Bridges that do not completely comply with today's more rigorous standards are technically called Obsolete. A fair amount of obsolescence in structures built 40 or more years ago should be expected and is normally acceptable.

The NBIS requires all structures that are 20 feet and longer in length as defined below to be inspected at least bi-annually. A few may require more frequent inspection depending upon condition and some structures may need special inspections such as underwater or fracture critical inspections. In Texas these inspections are all performed by highly trained and qualified TxDOT approved inspectors. TxDOT gathers this federally mandated information for the entire state to assure consistency across the numerous agencies and jurisdictions involved.





NBIS Structure Length

Figures 1a-d: NBIS Bridge Structures of 20' and Greater (inspected by TxDOT)





#### **General Bridge Conditions**

There are currently no bridges rated in Poor or Serious condition (<50) and only 6 bridges in Fair condition (50-60). The rest are all rated in Satisfactory or better condition with an SR at or above 60. And none are considered Deficient. Many of the lower rated bridges have received maintenance in our annual bridge maintenance contract, our CIP program, or are proposed for replacement in the near future. More than half of the structures in Fair or Satisfactory condition have received some type of maintenance in the last 10 years. A listing of our 44 lowest rated structures having a sufficiency rating below 70 (Good) is included for reference at the end of this document in Appendix A. In summary, having all of Austin's structures in Satisfactory or better condition represents a successful management program and protection of these valuable and critical infrastructure assets.

It should be noted that there is a clear trend in the age of our structures. Almost 30% of the inventory or 117 of these 412 bridges are past their currently accepted design life of 50 years. Fortunately, most of them are still performing quite well. But with increasing age and increasing traffic levels and loadings, these bridges will experience a faster rate of deterioration and eventually need more maintenance and rehabilitation. A long term plan for funding replacement structures must also be considered.



#### **Bridges with Load Posting Limits**

In the most recent inspection reports we have from the summer of 2016, TxDOT asked that we load post two bridges where the structure requires a load limit lower than the maximum legal allowable loading:

- 4th over Waller Creek
- Circle S over Boggy Creek

The structure over Waller Creek at 4th Street is an old bridge in downtown that Capital Metro Transit Authority is going to replace to support their Red Line light rail service. Circle S over Boggy Creek is a very old bridge that had abutment support problems until we added a straddle bent. Additionally, the very old reinforced concrete deck is now considered to have a restricted load carrying capacity. This indicates that we should enforce a load posting restriction.



#### **Bridge Management Information System**

The Bridge Management Information System (BMIS) mission is to "Improve organized knowledge of the condition of our bridge system which can be used to prioritize or optimize a plan for bridges needing preventive or repair maintenance, rehabilitation, or replacement in order to keep the City of Austin's bridges in a good serviceable condition for the traveling public."

In the past the City managed its bridge inventory through the Street and Bridge district supervisors on a repair as needed basis. This method had no systematic ability to forecast future rehabilitation or replacement needs or schedule preventive maintenance which would extend the useful life of these structures. The purpose of a BMIS is to transform bridge management from the reactive mode to a proactive one. We have detailed inspection files on each bridge with biannual ratings back to 1988. The data in these files is currently being used to determine maintenance priorities. A formal BMIS database system will be selected and recommended for purchase in 2019.

This BMIS will allow us to better manage our infrastructure assets and move from being reactive to being proactive. The BMIS will allow for modeling, analysis, and planning for this aging infrastructure. Our future bridge needs can then be more accurately forecast and projected for both maintenance, rehabilitation, and long term capital planning and budgeting for bridge replacements.

#### Maintenance, Rehabilitation, and Replacement

There are no bridges that currently have an SR rating in the poor range. All structures previously rated poor have been improved by our Capital Improvements Program (CIP) and TxDOT Off-System Bridge Program. In 2003 we established a steady budget for bridge maintenance from the Transportation Fund. Since then we have steadily contracted for repairs and preventative maintenance on 10 to 12 bridges each year.

#### **Pedestrian Bridges**

In addition to the above list the City is also responsible for maintaining pedestrian bridges and structures that are part of the Mobility Trail system. The following lists the pedestrian structures that are in immediate need of major rehabilitation or replacement:

#### Deficient Pedestrian Bridges

- Landon Lane at Lee Elementary
- 49th Street at Woodview over Shoal Creek
- Bethune Avenue
- Pecan Grove Road at Alameda Drive
- 2 bridges on Johnson Creek Mobility Trail
- Sparks at 31st Street
- Barton Parkway

#### Other Structures in the Right of Way

In addition to the bridge structures the City is also responsible for maintenance of retaining walls, special structures, guardrails, and embankments next to roadways and trails. There are currently needs identified for major maintenance along the Shoal Creek Trail and locations along roadways such as Hart Street. Recently constructed trails and structures like The Boardwalk and the Southern Walnut Creek Trails include over a mile of elevated structures and multiple pedestrian bridges within each system. Substantial facilities like these and others will require budgeting of adequate funding for future maintenance and repairs.

#### **Capital Program Needs**

Two projects stand out as critical needs in this area: the Emmett Shelton Bridge on Redbud Trail over the Lady Bird Lake and the William Cannon Drive MSE Walls.

The Emmett Shelton Bridge is critical to Austin because of the Ullrich Water Treatment Plant and our agreement with the City of Westlake. All heavy truck traffic must use this bridge to service the Ullrich plant. The bridge was studied in 1997 by CFX Engineering to determine its capacity for handling this continued heavy truck traffic, the construction traffic for the Ullrich expansion projects, and its remaining life in general. A structural enhancement project in 1998 was executed to give us about another 10 years; however, this period has elapsed. The structure is due for replacement as a critical link across the river serving both Austin and Westlake. The preliminary estimate for this project is \$50 Million.

The William Cannon Drive mechanically stabilized earth (MSE) walls have been a concern since their construction in the 1980s. Excessive movement in the panels was noted right after completion and has been monitored over the years. The situation stabilized for about 20 years, but began moving and failing again a few years ago. The overpass roadway started cracking severely and required major repairs. The design is being funded by 2006 and 2010 bond funding, but construction will have to be funded by a new bond program. The preliminary estimates for these projects are \$5 Million for the west end and \$5 Million east end.

Other bridge projects that are being recommended over the next 5 years are Barton Springs Rd Bridge over Barton Creek (\$9 M), Delwau Lane Bridge over Boggy Creek (\$5 M), Slaughter Ln MSE Walls (\$7.3 M), Riverside Drive Retaining Wall (\$1.5 M), and continued miscellaneous Minor Bridge & Culvert funding for smaller structures (\$5 M).

#### Emerging Issue – Maintenance of Culverts and Small Structures

There are about 1,100 smaller structures (less than 20') without inspection records and condition assessments. These are typically pipes and smaller culverts that allow drainage water to cross under the roadway. Fortunately, these smaller structures represent a much lower risk than larger bridge structures. They are less like to experience severe flood damage and the consequence of any failure is much lower. Historically, a relatively small Minor Bridge and Culvert capital program has been used to address the more substantial problems that occasionally occur with these smaller structures.

While we have detailed condition reports on all major structures, we only have locations and limited information on all of the smaller bridge, culvert, and pipe structures. Although much less critical, we will establish a routine inspection of these minor structures as well. This will allow us to maintain a comprehensive inventory, condition assessment, and ratings for all of our transportation structures.

Street and Bridge Operations has developed a map of all bridges and culverts where water crosses the right of way in GIS. This additional set of smaller bridges and drainage crossings will have to be inspected by City staff because they are not included in TxDOT inspections. We intend to initiate and complete this initial condition assessment and data collection over the next three years.

#### Maintenance Responsibility

These smaller culvert crossing structures within the right of way are both part of the roadway and at the same time part of the drainage infrastructure. PWD will maintain the structural integrity of the culvert or pipe itself including headwalls, railing, and pavement. Watershed will clean debris out the culverts to assure drainage flow and maintain vegetation control in the culvert entrance and exit areas.

#### Small Bridge & Culvert Structures – Description of Inventory Elements

Small pipes and culverts crossing the right of way have far fewer elements than typical bridge structures and are much less complicated. Minimal attributes will be required for data collection all of the simple pipe and box culverts. The following pictures of some pipe crossings show how few attributes are needed to adequately describe and characterize these simple structures.

These small structures are typically comprised of one or more buried pipes/culverts, minor safety systems (guardrailing), and small entrance and exit headwalls/aprons. These structures have no decks, superstructures, substructures, or underlying channels. Conveyance of water under the roadway is entirely contained within the pipes. Also, no bridge signage, approach slabs, or any other special roadway elements are usually needed. The roadway pavement over top is typically separate and will be maintained as an integral part of the street itself. It will not be considered part of the small bridge structure – unless it has a true deck and superstructure which very few do.

Alternatively, any small structures that are true bridges having suspended decks and superstructures spanning from abutments or end walls will be inspected and rated in more detail similar to large bridges over 20 feet in length on a two cycle.

#### **Typical Data Needed for Most Small Structures**

#### Structure:

- location,
- structure type [B, C, P],
- street/facility carried,
- feature crossed,
- year built,

#### Images:

- side-oblique of full site (1),
- both sides (2),
- roadway, both directions looking toward crossing (2),

#### Pipe(s):

- type,
- material,
- size,

- last repaired/rehab,
- pipe length,
- skew,
- paved width,
- flow width
- any special/unusual detail pictures
- dimensions/spacing,
- number of pipes,
- condition

#### Additional Data Needed for Some Small Structures (as appropriate)

#### Slope Protection/Riprap:

• protection - type, material, dimensions/area, condition

#### Headwall(s):

• pipe end and protection – type, material, dimensions, condition

#### Railing:

- guardrailing,
- parapet walls,
- bridge railing type, material, dimensions, condition

#### Safety End Treatments:

- turn downs,
- special end treatments type, material, dimensions, condition

#### Signs:

- bridge corners,
- edge markers sign designation, size, sign mount, condition

#### Typical Box Large Culvert Structures

(Inspected by TxDOT as NBIS bridges)



#### **Typical Small Box Culvert Structures**

(Not currently inspected as NBIS bridges)



Figures 2a, 2b: Small Structures of Less than 20' combined flow width are not inspected by TxDOT and reported to the National Bridge Inventory System (NBIS)



#### **Typical Small Pipe Culvert Crossing Structures**



## Standard Culvert Crossing in Good Condition

Small multi-pipe crossing with headwalls, guardrailing, and SETs. Abrupt roadway edge drop-off condition, although relatively shallow, warrants guardrailing protection.



#### Non-Standard Multi-pipe Culvert Crossing – Needs Maintenance

Small, extended multi-pipe crossing with no roadway elements at all. Pipe ends are far outside the roadway clear zone. Shallow bury, small diameter pipes only create a shallow edge dropoff which does not cause a concern for the roadway.



#### Non-Standard Pipe Culvert Crossing – Needs Improvement

Small single pipe crossing with no roadway elements at all. Pipe end is within the roadway clear zone and should probably be protected or the slope extended and improved. Deeper small diameter pipe creates an edge drop-off which may cause a concern for the roadway.

### Appendix A

| No. | Street Carried       | Feature Crossed               | Location                           | SR   |
|-----|----------------------|-------------------------------|------------------------------------|------|
| 1   | E 5th St EB          | Waller Creek                  | 0.10 mi E of Red River St          | 52.1 |
| 2   | S 1st Street         | Williamson Creek              | 0.9 mi S of SH 71                  | 57.8 |
| 3   | S 1st Street         | Williamson Creek              | 1.0 mi S of SH 71 (Ben White Blvd) | 58.2 |
| 4   | E Riverside Dr       | Country Club Creek Branch     | 3.7 mi E of Loop 275               | 58.4 |
| 5   | Vertex Blvd          | Onion Creek                   | 2.7 mi E of IH 35                  | 58.7 |
| 6   | S Pleasant Valley Rd | Colorado River - Longhorn Dam | 0.14 mi S of Cesar Chavez St       | 58.8 |
| 7   | N Lamar Blvd         | Shoal Creek                   | 0.10 mi S of W 15th St             | 60.2 |
| 8   | Circle S Rd          | Boggy Creek                   | 0.90 mi S of William Cannon Dr     | 60.8 |
| 9   | S 1st Street         | Williamson Creek              | 1.15 mi S of SH 71                 | 61.6 |
| 10  | Baythorne Dr         | Onion Creek                   | 2.7 mi W of IH 35                  | 61.9 |
| 11  | E Riverside Dr       | Willow Creek                  | 1.9 mi E of Loop 275               | 62.8 |
| 12  | W Stassney Ln        | Williamson Creek Branch       | 0.8 mi W of S 1st St               | 63.1 |
| 13  | E Stassney Lane      | Williamson Creek              | 0.25 mi E of IH 35                 | 63.7 |
| 14  | Pleasant Valley Rd   | Williamson Creek              | 0.1 mi N of William Cannon Dr      | 64.0 |
| 15  | E 7th St EB          | Tillery St & CMTA Rail        | 1.6 mi E of IH 35                  | 65.1 |
| 16  | E 7th St WB          | Tillery St & CMTA Rail        | 1.6 mi E of IH 35                  | 65.1 |
| 17  | E Riverside Dr       | Blunn Creek                   | 0.30 mi W of IH 35                 | 65.5 |
| 18  | W 15th St            | Shoal Creek/Lamar Pkwy        | 0.20 mi W of West Ave              | 65.6 |
| 19  | W 45th St            | Shoal Creek                   | 0.5 mi W of Burnet Rd              | 65.7 |
| 20  | Slaughter Ln         | Slaughter Creek Tributary     | 0.14 mi W of Brodie Ln             | 66.0 |
| 21  | S Congress Ave       | Lady Bird Lake                | 0.1 mi S of Cesar Chavez St        | 66.1 |
| 22  | Redbud Trail         | Colorado River Relief         | 0.30 mi W of Lake Austin Blvd      | 66.3 |
| 23  | E 45th St            | Waller Creek                  | 1.9 mi W of Airport Blvd           | 66.8 |
| 24  | Cameron Rd           | Little Walnut Creek Branch    | 0.50 mi N of US 183                | 66.8 |
| 25  | S 1st St             | Boggy Creek                   | 1.0 mi S of William Cannon Dr      | 66.9 |
| 26  | Redbud Trail         | Colorado River                | 0.20 mi W of Lake Austin Blvd      | 67.1 |
| 27  | Barton Springs Rd    | Barton Creek                  | 0.70 mi E of Loop 1 (MoPac)        | 67.3 |
| 28  | E 15th St            | Waller Creek                  | 0.27 mi E of Congress Ave          | 67.7 |
| 29  | Metric Blvd          | Walnut Creek                  | 0.70 mi S of Parmer Ln             | 67.8 |
| 30  | E Riverside Dr EB    | Country Club Creek            | 1.4 mi E of IH 35                  | 67.9 |
| 31  | E Riverside Dr WB    | Country Club Creek            | 1.4 mi E of IH 35                  | 67.9 |
| 32  | E Stassney Lane      | Draw                          | 2.2 mi SE of IH 35                 | 67.9 |
| 33  | E Stassney Lane      | Draw                          | 2.8 mi E of IH 35                  | 67.9 |
| 34  | Manchaca Rd          | Williamson Creek              | 0.70 mi S of US 290                | 68.0 |
| 35  | Old San Antonio Rd   | Slaughter Creek               | 0.20 mi W of IH 35                 | 68.3 |
| 36  | Manor Rd             | Little Walnut Creek           | 0.5 mi W of US 183                 | 68.6 |
| 37  | Payton Gin Rd        | Little Walnut Creek           | 0.1 mi W of Lamar Blvd             | 68.7 |
| 38  | Gracy Farms Ln       | Branch Walnut Creek           | 0.35 mi E of Burnet Rd             | 68.7 |
| 39  | S 1st St             | E Bouldin Creek               | 0.5 mi S of Barton Springs Rd      | 69.0 |
| 40  | W 51st St            | Waller Creek                  | 0.40 mi E of Lamar Blvd            | 69.1 |
| 41  | Mt Bonnell Rd        | Dry Creek                     | 0.4 mi S of FM 2222                | 69.1 |
| 42  | S 1st St             | E Bouldin Creek               | 0.1 mi S of Barton Springs Rd      | 69.7 |
| 43  | Walnut Creek Cross   | Walnut Creek Branch           | 0.02 mi W of Park 35 Cir           | 69.8 |
| 44  | W Oltorf St          | W Bouldin Creek               | 0.15 mi E of S Lamar Blvd          | 69.9 |

### Listing of 44 Lowest Rated Bridge Structures with Sufficiency Ratings (SR) below 70

#### Background

Mechanically Stabilized Earth (MSE) walls were developed as an extremely efficient technology for bridge embankments. They are still widely in use today. The William Cannon Drive Overpass embankment was placed during the first generation of MSE wall designs in 1983. William Cannon Drive is a major 6-lane divided arterial roadway. And as such, it is a critical roadway network asset serving south Austin.

The west end of this large overpass structure exhibited movement immediately after its initial construction. Several experts were consulted for an evaluation of the situation at that time. Minor construction quality issues and marginal materials seemed to be the cause of these early concerns. Ultimately, the City decided to monitor the situation rather than reject the work entirely. The situation did stabilize and showed no significant movement for about 10 years.

Near the end of the 1990s movement was realized again by large cracks appearing in the roadway above. The MSE walls on both sides of the west approach embankment to the bridge have experienced movement and deflections that has caused noticeable separation between the sidewalk and the street. This movement has also triggered voids and major cracking in the roadway surface. So, after 20 years of monitoring and growing concerns over its condition, a recommendation and design was requested for the permanent repair of the west end.

A substantial amount of geotechnical and background information was gathered for the design of the MSE walls on the west end of the William Cannon Overpass. This culminated in the preparation of an engineering report of findings and recommendations prepared by Dannenbaum Engineering Corporation in 2003. Full construction plans for both ends are also available.

The design that was tentatively selected for the west end of this overpass in 2003 was to extend the bridge for an additional span to lower the necessary height of the remaining MSE walls. However, we were not able to pursue this alternative at that time due to its very high cost. Other options such as soil nails and other geotechnical solutions were eliminated at that time due to feasibility concerns. Fortunately, it has come to our attention that a lot has changed in the industry since then. Numerous MSE walls of this generation have been repaired using several newer and more cost-effective techniques. For example, soil nailing technologies and equipment have advanced tremendously during this period. For this reason we want to reconsider all currently feasible options for the safe, efficient, timely, and cost-effective repair of this overpass embankment.

We are now requesting recommendations and design for the east end of the overpass since the east end was similarly constructed. We now know that there were minor flaws in the industry's design practices for the first generation of these walls which further compounded our construction issues. If a more practical, beneficial, and affordable repair method can be developed; we will recommend constructing repairs to the west end using the same method. Overall, it seems prudent to retrofit both ends to assure the overpass and bridge remain stable, safe, and reliable for their remaining service life of 50 or more years.

#### **Alternative Solutions**

We want to consider several alternatives with the ultimate objective of a balance of safety, reliability, minimizing citizen disruption to the degree possible, and cost. The current options appear to be

- 1) Entirely Reconstruct the MSE Walls
- 2) Extend the Bridge to Replace Distressed MSE Walls
- 3) External Soldier Piles/Walls
- 4) GeoPiers/Stiffen Soil Mass and Foundation Soils
- 5) Soil Nailing

However, any other appropriate solutions should be investigated and considered at the consulting engineer's discretion. As stated before, we want to reconsider options that were previously eliminated as infeasible in the 2003 recommendations.





#### **Concerns About Other MSE Wall Locations**

We only have a few walls that were similarly designed and constructed like those at the William Cannon overpass and most are not as large or as critical. However, since the Slaughter Lane overpass is very similar, it too should be analyzed and considered for rehabilitation. Fortunately, this may be the only other major structure of this type on our arterial roadway network.

Furthermore, embankment wall technology has improved a lot since the early 1980s. Specifically, changes have since been made to the design standards for MSE walls which have corrected these problems. TxDOT has experienced similar issues and is still retrofitting some of their older walls today. Regardless, there are new MSE walls all over the Austin area on IH-35, US 183, US 290, MoPac, and Hwy 71. However, these new retaining walls and embankments are far superior and constructed much more conservatively and reliably than the older walls.

# Delwau Lane Bridge over S Boggy Creek

Current Bridge: 43' waterway opening 30' wide deck

100 Yr Floodplain: 470' wide















## Delwau Lane Bridge over S Boggy Creek

## **October 1998 Flood Damage**



## Delwau Lane Bridge over S Boggy Creek

## August 1999 Flood Damage & Massive Voids













# Delwau Lane Bridge over S Boggy Creek July 2006 Abutment Undermining & Repairs after June Flood













## Delwau Lane Bridge over S Boggy Creek

## July 2006 Abutment Undermining & Repairs after June Flood (cont'd)











## Delwau Lane Bridge over S Boggy Creek January 2007 Flood Damage















# Delwau Lane Bridge over S Boggy Creek

## 2008 Northeast Bank Scouring after High Flow Event





### 2010 Additional Northeast Bank Scouring after High Flow Event



## Delwau Lane Bridge over S Boggy Creek May 2015 Flood Damage & Concrete Abutment Fill





### April 2016 Flood Damage & Mortared Rubble Repair









### Condition Report on the Emmett Shelton Bridge on Redbud Trail over Lady Bird Lake

Street & Bridge Operations September 28, 2017

#### Background

 The Emmett Shelton Bridges on Redbud Trail over Lady Bird Lake were originally built in 1948 making the structures 68 years old. There are actually two separate bridges. A 400 foot bridge spanning from the Austin side on the East bank to Redbud Isle and a 100 foot bridge to the West bank on the Westlake Hills side.



- The structure is used by more than 13,000 vehicles per day as a critical commuter link between Austin and Westlake Hills. And, a primary problem with these obsolete structures at this point is that they are too narrow for current traffic volumes.
- The bridges are critical to the servicing and operation of the Ullrich WTP facility because of truck traffic restrictions and concerns with all other routes out of the plant. All other routes have been well studied, considered, and ultimately rejected over the years.
- Interlocal Agreement (Resolution 871005-02) with Westlake Hills was executed on October 5, 1987 calling for the City of Austin to set aside \$4 Million from a proposed 1989 bond election for roadway and bridge improvements on Redbud Trail serving the Ullrich WTP. In the event the citizens of Austin rejected such a program, the two cities would re-evaluate the situation and potential sources of funding. However, no bond election was held which made the specific language ineffectual. Thus, the intent and need remains to negotiate a new resolution to these issues.
- The bridges also have very narrow, substandard sidewalks serving as the Western end of the Lance Armstrong Bikeway (LAB) and are the only pedestrian and bike route to the popular parks destination Redbud Isle.

#### **Design Issues**

- Many bridges constructed in this era were only designed for the lighter truck loadings of that era and a 50 year design life. And, trucks in the 1940s were about half the weight of today's trucks. Therefore, these structures have seen more than twice the design loadings they were designed for and; therefore, they are well beyond the anticipated design life of these highly used bridges.
- The Ullrich WTP traffic has further and substantially increased the truck loadings beyond normally expected traffic and well beyond designed loadings. There were 13 sludge and chemical trucks per day weighing between 70,000 and 77,000 lbs gross in 1996. The 6 year plant expansion project added construction traffic and increased the operational trucking demands with the increased Ullrich WTP capacity.

#### **Bridge Condition**

- Bi-annual inspections by professional engineers are done for the Fed's National Bridge Inventory System (NBIS) through TxDOT's Bridge Inspection and Appraisal Program (BRINSAP). These inspections are done by highly trained engineers that meet TxDOT's stringent requirements for bridge experience.
- A thorough and rigorous analysis and load testing by CFX Engineering in 1997 was consistent with an all-time low SR of 36.4 (out of 100) and established a limited remaining useful life. No recent or further special testing, inspections, or analysis have been done beyond the CFX Engineering study referenced below under possible safety concerns and the normal TxDOT inspections.
- The last TxDOT inspection was done in 2010 and resulted in a Sufficiency Rating (SR) of 67.0 which is a rating of "Fair". And, TxDOT ratings have been consistently around 67 since our rehabilitation project was completed in 1999. Despite this seemingly acceptable rating, we are concerned that the somewhat cursory visual inspections performed are highly overrating the structural capacity of this bridge (see last three points under Safety Concerns section). The interim enhancements designed by CFX and made in 1999 were only intended to extend the usable life of the structure until we could plan to replace the bridge in about 2006 contingent upon availability of capital funding.

#### Safety Concerns

- The two bridges on Redbud Trail collectively known as the Emmett Shelton Bridge are still considered safe at this time and remain open to all legal truck loads. However, the bridges are relatively old and the exact end of their life cycle is impossible to predict with precision. Therefore, bridge inspections have been conservative to assure that the existing structures are performing well enough to keep them open with confidence.
- Exact prediction of when a bridge may reach the end of its useful life is impossible, however, a very detailed analysis and actual load testing by CFX Engineering lead by Dr. Ramon Carrasquillo of the University of Texas was performed in an attempt to predict the best possible estimate of the remaining service life (RSL).
- The CFX report showed that this structure was already very close to the end of its useful life in 1997 before the Ullrich WTP expansion construction project. Therefore, the bridge deck was structurally modified in late 1998 to enhance its load carrying capacity. The theoretical "grace" period of another 8 years calculated by CFX expired in 2008.
- This bridge may become critically and structurally deficient over the next few years and is at high risk of steel girders reaching their fatigue failure life (too many millions of loadings). We will attempt to monitor the bridge closely recognizing that these types of failures can be very serious and require quick remedial action without much warning. Fortunately, this structure has redundant girders meaning that a sudden catastrophic collapse as seen elsewhere in the nation is extremely unlikely.
- Severe load restrictions, limited number of allowable truck loadings per day, lengthy detours and closures are a distinct possibility if any excessive distresses or damages are noted over the next few years.
- Furthermore, a previous study done for the LCRA as a tabletop study of a "rain bomb" event showed that the Redbud trail Bridge would be about 6" under water in a 100-year flood event and possibly could stay out of service for up to 3 days during such events. This period could even be longer if severe scouring were to compromise the stability of the bridge foundations as a result of flood waters or LCRA flood water releases.

|           |                            | Top of Bridge/ | Flood      | Depth of Water      | Time        |
|-----------|----------------------------|----------------|------------|---------------------|-------------|
| Lake      | Crossing                   | Rdwy or Dam    | Crest      | Above/Below         | below Water |
|           | _                          | Spwy Elev.     |            |                     |             |
|           |                            | (feet msl)     | (feet msl) | (feet)              | (days)      |
| Lake      | Downstream of Starcke Dam  | 738.0          | 725        | -13.0               | N/A         |
| Travis    | Upstream of Mansfield Dam  | 714.5          | 722        | 7.5                 | 3 days      |
| Lake      | RR 620                     | 650.0          | 512.8      | -137.2              | N/A         |
| Austin    | Low Water Crossing         | 500.7          | 511.24     | 10.5                | +30 days    |
|           | SH 360                     | 536.5          | 495.67     | -40.8               | N/A         |
|           | Upstream of Tom Miller Dam | 492.8          | 492.8      | 0.0                 | N/A         |
|           | Redbud Trail               | 445            | 445.6      | 0.6                 | 3 days      |
|           | Mopac                      | 485            | 444.1      | -40.9               | N/A         |
|           | Lamar                      | 450            | 441.3      | - <mark>8</mark> .7 | N/A         |
| Town Lake | S 1st                      | 446            | 440.6      | -5.4                | N/A         |
|           | Congress                   | 450            | 440.5      | -9.5                | N/A         |
|           | IH 35 Frontage Road        | 454            | 439.8      | -14.2               | N/A         |
|           | IH 35 Mainlanes            | 458            | 439.8      | -18.2               | N/A         |
|           | Longhorn Dam               | 472.5          | 438.6      | -33.9               | N/A         |
|           | US 183(SB)                 | 459            | 436.8      | -22.2               | N/A         |
| Travis    | US 183(NB)                 | 453.3          | 436.8      | -16.5               | N/A         |
|           | FM 973                     | 416            | 415.8      | -0.2                | 2.5 days    |

Transporation Impacts for Flood Drill

Estimated time under water does not imply that the bridge will not be damage or out of service longer. The positive depth of water gives the depth over the approach roadway or bridge deck. Bridges also may be cut off due to storm drains backing up and are not accounted for in this table.

#### **Proposed Bridge**

- A single long-span bridge has been proposed as a replacement for the two bridge setup. Major roadway geometry changes to Redbud Trail have also been proposed to benefit our Ullrich WTP truck traffic and all users.
- A 10 foot wide sidewalk and bike path has also been proposed as part of this structure to extend the LAB and usable sidewalk along Lake Austin Blvd to Redbud Isle.
- The proposed bridge is also intended to drastically improve the alignment of the bridge approach roadway on Redbud Trail by smoothing out the tight curves at the west end of the existing bridge. This can be seen in the image of the proposed roadway (green lines) and bridge alignment (red lines) on the following page. There have been numerous complaints and many documented accidents in the tight S-curves and at the "kink" in the road at the west end of the bridge. These both represent severely obsolete roadway geometries which are considered intolerable in modern roadway design.
- The sharp curves on the west approach are made worse by the rather steep slope of the roadway approaching the bridge from the west descending from the top of the bluff down to the existing bridge crossing level. In recognition of this fact, the proposed bridge is also intended to improve the vertical profile of this crossing by raising the elevation of the entire bridge by about 25 feet. This allows for making the approach roadways less steep, improves use by City of Austin sludge trucks, and places the bridge deck entirely above of all predictable flood events.

#### Bridge Replacement Justification

The Emmett Shelton Bridge on Redbud Trail over Lady Bird Lake is a critical link between Austin and Westlake Hills and is now 68 years old. The bridge is critical for servicing and operating the City of Austin Ullrich Water Treatment Plant facility. Many bridges constructed in this era were designed for the much lighter truck loadings of that time and a 50 year design life. The Ullrich WTP traffic has substantially increased the truck loadings beyond the normally expected traffic and well beyond these design expectations – possibly more than twice the anticipated design loadings and well beyond its anticipated design life.

The exact prediction of when a bridge will begin to fail is impossible, so a cautious approach must be taken to bridge replacement. Bridge inspections will continue to assure that the bridge is performing adequately to keep open with confidence, but caution is advised. A very detailed analysis and load testing by CFX Engineering and lead by Dr. Ramon Carrascillo of the University of Texas was performed in 1997 to predict an estimate of the bridge's remaining service life (RSL). The CFX report showed that this structure was very close to the end of its useful design life at that time. As a result, the bridge deck was structurally modified to enhance its load carrying capacity to theoretically increase the bridge's RSL another 8 to 10 years and through the planned Ullrich WTP expansion project. Although substantial safety factors are used in the design of bridges, we need to push forward now with the renewal of this structure.

Despite the deck improvements in 1998, the bridge is still at risk of its steel girders reaching fatigue failure (too many millions of load applications) over the next few years and becoming structurally deficient. If excessive distresses or damages are noted by inspections; severe load restrictions, limited allowable truck loadings per day, and lengthy detours or even closures are a distinct possibility. Unfortunately, any such limitations on bridge loadings would also jeopardize the required daily Ullrich WTP traffic.

There are also some serious waterway related concerns for the current structure. A study done for the LCRA as a tabletop study of a "rain bomb" event showed that the Red Bud Trail Bridge would be about 6" under water in a 100-year flood event and possibly could stay out of service for up to 3 days during such events. This period could even be longer if severe scouring were to compromise the stability of the bridge foundations as a result of flood waters or LCRA flood water releases.







#### Background

The Barton Springs Road Bridge over Barton Creek is critical to the welfare of the daily commuters because it provides connection for vehicular access to several major roads and communities. Barton Springs Road Bridge over Barton Creek is located near the intersection of Robert E Lee Road and Barton Springs Road. The bridge is obsolete and requires rehabilitation or replacement. The bridge was originally built in 1925 and was expanded on one side in 1946. The current bridge is 212' long and 58'-8" wide. Structurally it appears to be in fair condition; however, the deck width and geometry are obsolete. The bridge is currently a bottle-neck for the enhancement of all modes of travel on Barton Springs Road approaching Zilker Park from the east.

In 2011, City staff within Street and Bridges Operations Division proposed a proof of concept design that would widen the bridge deck 30 feet to accommodate 2 - 6' sidewalks, 2 - 5' bicycle lanes, 4 - 10' travel lanes and a 15' median, thus matching the new cross section of Barton Springs Road established by the reconstruction of that roadway east of Robert E Lee Rd. This proof of concept was completed in-house and was not a formal report or document that was certified by an Engineering firm. The cross section of the newer, wider Barton Springs Road to the east that was completed in mid-2003 has been a mismatch to the older roadway because of the lack of a median, substandard sidewalks, and missing bike lanes on the bridge. Successive discussions amongst City staff in the Public Works Department and Austin Transportation Department confirmed the inadequacy of these elements in addition to many other aspects of this intersection in the decade since.

The selected engineering firm will serve as the bridge designer and will present options for the rehabilitation or replacement of the bridge that shall provide safe and efficient access for people, goods, and vehicles across Barton Creek. The bridge will meet current design standards for cars, trucks, pedestrians, and bicycles, and will serve for 100 years if replaced and 40 years if rehabilitated.

#### Scope of the Design

This project includes the complete replacement of the obsolete Barton Springs Road Bridge over Barton Creek located near the intersection of Robert E Lee Road. Construction of the Public Works portions of this complex project is anticipated to cost around \$4 Million.

The bridge is currently a bottle-neck for the enhancement of all modes of travel on Barton Springs Road approaching Zilker Park from the east. This project would double the width of the bridge deck to accommodate 2-6' sidewalks, 2-5' bike lanes, 4-10' travel lanes and a 15' median thus matching the new cross section of Barton Springs Road established by the reconstruction of that roadway east of Robert E Lee Road.

There are a large number of interrelated improvement needs at the intersection of Barton Springs Road and Robert E Lee including:

- Fairly complex bridge geometry including an immediate adjacent "T" intersection
- Realignment of traffic lanes to match new Barton Springs cross section east of Robert E Lee
- Structural sidewalk and bridge class railing along west side of Robert E Lee
- Expansion of the bike lanes across the bridge and through the intersection
- Large retaining wall and slope stabilization along the Umlauf property
- Sidewalk connectivity on the southeast corner
- Redesign of signalized traffic intersection
- Street drainage design
- Environmental protection of Zilker Park and Barton Creek
- Protection of the creek & trailside amenities below the bridge
  - o Hike & Bike Trail
  - Terraced Slopes & Plantings along the trails
  - o Zilker Park Train
- Multiple public and private utilities attached to the bridge

As such, this project directly affects multiple departments: SBO, NCD, PARD, ATD, WPD, AWU, and any private utilities attached to the bridge. The Director of Public Works intends to find funding for the design of the bridge replacement, approach roadways, bike lanes, sidewalk, street drainage, and retaining wall improvements. However, the other affected infrastructure departments will need to participate in the funding of the project to complete the design.







































### E 9th Street Bridge over Waller Creek



Bridge was closed during the bridge inspection on 3/30/2016 for construction of utilities related to new hotel at 9th and Red River.

#### ROADWAY VIEW

Looking East

NOTE: Bridge closed to house construction equipment for hotel construction at SE corner. Bridge open to pedestrian traffic.



SIDE VIEW

Looking Southwest

**Travis County** 

Bridge No.: B000-28-007

Date: Mar. 30, 2016

Clear Span Engineering, Inc. (F-711)

|          |                        | 000-28-007 0011 RT 1 | • Tex   | as Department of T           |                        |                                       | Ru                        | n Date: 4/11/2016     |
|----------|------------------------|----------------------|---------|------------------------------|------------------------|---------------------------------------|---------------------------|-----------------------|
|          | STATUS: Active         | 1 0                  |         | Bridge Divis                 | ion                    |                                       | /                         | /                     |
|          | TION STATUS: Approved  | R                    |         | PonTex Detail Brid           |                        | •                                     | FACILITY: E 9TH ST EB     |                       |
| <u> </u> | FEATURE: WALLER C      |                      |         |                              |                        | · · · · · · · · · · · · · · · · · · · | LOCATION: 0.42 MI E OF CO |                       |
| EM #     | DESCRIPTION            | VALUE                | ITEM #  | DESCRIPTION                  | VALUE                  | ITEM #                                | DESCRIPTION               | VALUE                 |
| ( 005-1  | ) STRUCTURE FUNCTION   | 1                    | ( 045-3 | ) NO.MIN APPR SPAN           |                        | (106-1                                | ) WIDENING CODE           | 0                     |
| (004-0   | ) CITY CODE            | 02100                | ( 046-0 | ) TOTAL NUMBER SPANS         | 1                      | (107-1                                | ) DECK STR TYPE MAIN SPAI |                       |
| ( 005-2  | ) HWY. SYS             | 31                   | (047-0  | ) TOTAL HORIZ CLR            | 064.0                  | · ·                                   | ) MAIN SPAN WEAR SF       | 188                   |
| ( 005-3  | ) RT. DESIGN           | 1                    |         | ) MAX.SPAN LENGTH            | 58                     | · ·                                   | ) STR.TYP MAJ APP SP      | N                     |
| ( 005-4  | ) HWY NO               | 0000                 | ( 049-0 | ) STR.LENGTH                 | 60                     | (108-2                                | ) MAJ.APP SPN WEAR SF     | NNN                   |
| 005-5    | ) ROUTE DIR            | 0                    | ( 050-1 | ) LEFT SIDEWALK              | 07.0                   | (107-3                                | ) STR.TYP MIN APP SP      | N                     |
| 005-6    | ) ROUTE SUFFIX         |                      | ( 050-2 | ) RIGHT SIDEWALK             | 07.0                   | (108-3                                | ) MIN.APP SPN WEAR SF     | NNN                   |
| 006-2    | ) CRIT. BRDG           |                      | ( 051-0 | ) ROADWAY WIDTH              | 046.0                  | 1 ·                                   | ) AADT TRK PRECENT        | 0                     |
| 010-0    | ) RT.MIN VERT CLR      | 99FT 99IN            | ( 052-0 | ) DECK WIDTH                 | 080.0                  |                                       | ) DESIGN NATION NETWORK   | ۰ ( ۱                 |
| 011-0    | ) MI-POINT             | 00.420               | ( 053-0 | ) VERT.CLR OV                | 99FT 99IN              | (111-0                                | ) PIER/ABUT PROTECT       |                       |
| 011-1    | ) MIPT DATE (PRI)      | 201404               | ( 054-1 | ) VERT.CLR REF FEAT          | N                      |                                       | ) NBIS BRIDGE LENGTH      | Υ                     |
|          | ) REF-MKR and DISP     |                      | ( 054-2 | ) VERT.CLR UND               | 00FT 00IN              | (113-0                                | ) SCOUR CRITI BRDG        | 5                     |
|          | ) MIPT DATE (IR)       |                      | ( 055-1 | ) LAT.CLR REF FEAT           | N                      | (113-1                                | ) SCOUR VULNERABILITY     | A                     |
| •        | ) BASE HWY NETWORK     |                      | ( 055-2 | ) RIGHT LAT CLEAR            | 99.9                   | (114-0                                | ) FUTURE AADT             | 2000                  |
|          | ) LRS INVENTORY        |                      | ( 056-0 | ) LEFT LAT CLEAR             | 00.0                   | ( 115-0                               | ) FUTURE AADT YEAR        | 2030                  |
|          | ) LRS SUB RT.          |                      | ( 058-0 | ) DECK COND                  | 7                      | (116-0                                | ) MIN.NAVIG VERT CLR      | Neither Deficient nor |
|          | ) LATITUDE (D / M / S) | 30° 16' 08.12"       | ( 059-0 | ) SUPERSTR COND              | 8                      |                                       | ) COST ORIGN CONSTR       |                       |
|          | ) GPS LAT (DEG.)       | 30.26892208          | ( 060-0 | ) SUBSTR COND                | 7                      |                                       | ) DEFI / OBSO / 10YR RULE | (Def./Obs.)           |
|          | )LONGITUDE (D/M/S)     | 97° 44' 06.40"       | ( 061-0 | ) CHANN-PROTECT              | 6                      | (121-0                                | ) SUFF.RATING (SR)        | 088.5                 |
|          | ) GPS LONG (DEG.)      | 97.73511014          | ( 062-0 | ) CULVERT                    | N                      | (122-0                                | ) X-REF. PRIMARY RT ID    | 8031140070            |
|          | ) COLLECTION METHOD    | 3                    | ( 063-0 | ) METHOD OPR.RATING          | 5                      | (123-0                                | ) X-REF STR FUNC PRIMARY  | 'RT 1                 |
|          | ) BYPASS LENGTH        | 2                    | ( 064-0 | ) OPERATIONAL RATING         | 236                    | (124-0                                | ) X-REF IR ID             |                       |
| 020-0    |                        | 3                    | ( 065-0 | ) RDWY APPR COND             | 67                     | 125-0                                 | ) X-REF STR FUNC IR       | Dan                   |
|          | ) MAINT. RESPON        | 04                   | ( 065-1 | ) METHOD INV.RATING          | 5-                     | 126-0                                 | ) DIST USE                | BOO                   |
|          | ) OWNER                | 04                   | ( 066-0 | ) INVENTORY RATING           | 227 -                  | (128-0                                | ) OV HEIGHT DAMAGE        | N                     |
|          | ) MAINT.SECT NO        | B0                   |         | ) STR. EVALUATION Structura  |                        |                                       |                           |                       |
|          | ) PROJECT TYPE         | 1                    | (068-0  | ) DECK GEOMETRY was miss     | ing (0) for SR 5       | - C                                   |                           |                       |
| •        | ) CONT / SECT / JOB    | 8031-14-000          | ( 069-0 |                              |                        | (008-4/                               | ) IR.CONTROL              |                       |
|          | ) FUNCT.CLASS          | 44                   | ( 070-0 | ) BRIDGE POSTING Calculation | on. 6 is Satisfactory. | 1008-5A                               | ) IR.SECTION              |                       |
|          | ) YR ORGIN BUILT       | 1968                 | (071-0  | ) WATERWAY ADEQUACY          | 6                      | (011-0A                               | ) IR.MILEPOINT            |                       |
|          | ) LANES ON STR         | 03                   | ( 072-0 | ) APPR RDWY ALIGN            | 8                      | (008-6A                               | ) IR.STR NO               |                       |
| 028-2    | ) LANES UNDER STR      | 00                   | (075-0  | ) TYPE WORK-REPLACE          |                        | ( 008-3A                              | ) IR.DUPL OVER            |                       |
| 029-0    | ) AADT                 | 1870                 | ( 076-0 | ) LENGTH IMPROVEMENT         | 0                      | ( 005-1A                              | ) IR.FUNCTION             |                       |
| 030-0    | ) YR OF AADT           | 2010                 | ( 088-0 | ) ST-FRAC-CRIT/STEEL         | NNNN                   | ( 005-3A                              | ) IR.DESIGNAT             |                       |
| 031-0    | ) DESIGN LOAD          | 0                    | ( 090-0 | ) LAST INSP(MMDDYYYY)        | 3/30/16 3/2472014      |                                       | 、) IR.HWY SYS             |                       |
| 032-0    | ) APPROACH WIDTH       | 40                   | (091-0  | ) DESIGNAT INSP FREQ         | 24                     | ( 005-4A                              | ) IR.HWY NO               |                       |
| 033-0    | ) MEDIAN               | 0                    | ( 092-1 | ) FRACT/CRITI DETAIL         | N                      | ( 005-5A                              | ) IR.DIR                  |                       |
| 034-0    | ) SKEW                 | 0                    |         | ) UNDERWATER INSP            | N                      | ( 005-6A                              | 、) IR. ROUTE SUFFIX       |                       |
| 035-0    | ) STR.FLARED           | 0                    | •       | ) OTHER SPECIAL INSP         | N                      | ( 011-2A                              | ) IR.REF-MKR / DISP       |                       |
| 036-0    | ) TRAF.SAFETY FEAT     | 0000 -0000           |         | P) FRACT/CRIT FREQUENCY      |                        |                                       | ) IR.HORIZ CLR            |                       |
| 037-0    | ) HISTORICAL SIGNIF    | 3                    |         | ) UNDERWATER FREQUENCY       |                        |                                       | ) IR.RT.MIN VERT CLR      |                       |
|          | ) NAVIG CNTL           | 0                    |         | ) OTHER SPECIAL FREQUENCY    |                        |                                       | ) IR.BASE HWY NETWK       |                       |
| 039-0    | ) NAVIG VERT CLR       | 0                    |         | ) FRACT/CRITI (MMYYYY)       |                        |                                       | ) IR.LRS HWY              |                       |
| 040-0    | ) NAVIG HORIZ CLR      | 0                    |         | ) UN/WATER INSP (MMYYYY)     |                        |                                       | ) IR.LRS SUB RT           |                       |
| 041-0    | ) OPER.STATUS          | KA                   |         | ) OT/SPEC.INSP (MMYYYY)      |                        |                                       | ) IR.BYPASS LGTH          |                       |
| 041-1    | ) LOAD TYPE            | N                    |         | ) BDG IMPROVE COST           |                        |                                       | ) IR.TOLL                 |                       |
| 041-2    | ) LOAD IN 1000 LBS     | NNN                  | •       | ) RDWY IMPROVE COST          |                        | · ·                                   | ) IR.FUNCT CLASS          |                       |
| 042-0    | ) TYPE SERVICE         | 15                   | •       | ) TOTAL PROJECT COST         |                        |                                       | ) IR.AADT                 |                       |
| 043-1    | ) MAIN SPAN TYPE       | 1131                 | •       | ) YR IMPROVE COST EST        |                        | ( 030-0A                              | ) IR.YEAR OF ADT          |                       |
| 043-2    | ) MAJ.APP SPAN TYPE    |                      |         | ) BORDER BRIDGE              |                        | (100-04                               | ) IR.DEF HWY DESIGN       |                       |
|          | ) MIN.APP SPAN TYPE    |                      |         | ) BORDER STR NO              |                        | ( 101-04                              | ) IR.PAR STR DESIG        |                       |
|          | ) CULVERT TYPE         |                      | ( 100-0 | ) DEFENSE HWY DESIGN         | 0                      | ( 102-04                              | ) IR.DIR OF TRAF          |                       |
|          | ) TUNNEL TYPE          |                      | ( 101-0 | ) PARALLEL STR DESIGN        | N                      | ( 103-0A                              | ) IR.TEMP STR DESIGN      |                       |
|          | ) SUBSTR MAIN SPAN     |                      | ( 102-0 | ) DIR OF TRAFFIC             | 1                      | (104-04                               | IR.N H S                  |                       |
|          | ) SUBSTR MAJ APP SPAN  |                      | ( 103-0 | ) TEMP STR DESIGN            |                        |                                       | ) IR.AADT TRK PCT         |                       |
|          | ) SUBSTR MIN APP SPAN  |                      | ( 104-0 | )NHS                         | 0                      | ( 110-0A                              | ) IR.DESIG NAT NETWK      |                       |
|          | ) NO.MAIN SPAN         | 1                    | ( 105-0 | ) FED LANDS HWY              | 0                      | ( 114-0A                              | ) IR.FUTURE AADT          |                       |
| •        | ) NO.MAJ APPR SPAN     |                      | (106.0  | ) YR RECONST                 | 0                      | ( 115-0A                              | ) IR.YEAR OF AADT         |                       |